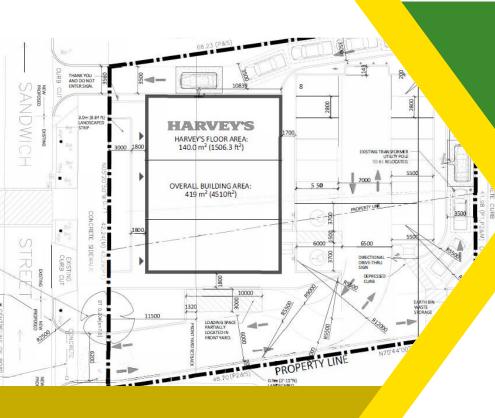
131 & 135 Sandwich Street



Transportation
Impact
Assessment



131 & 135 Sandwich Street Transportation Impact Assessment

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1 Introduction

This Transportation Impact Assessment (TIA) and Parking Justification has been prepared to support the proposed development located at 131 & 135 Sandwich Street in Amherstburg, Ontario. The development land is currently designated as a General Commercial Zone. The site is proposed to include a 140 m² drive-through restaurant (anticipated to be a Harvey's), and 247 m² allocated towards two take-out restaurants. A total of 15 parking spaces, nine queuing spaces, and one loading space are proposed. The anticipated use of two adjacent municipal parking lots will provide additional parking areas.

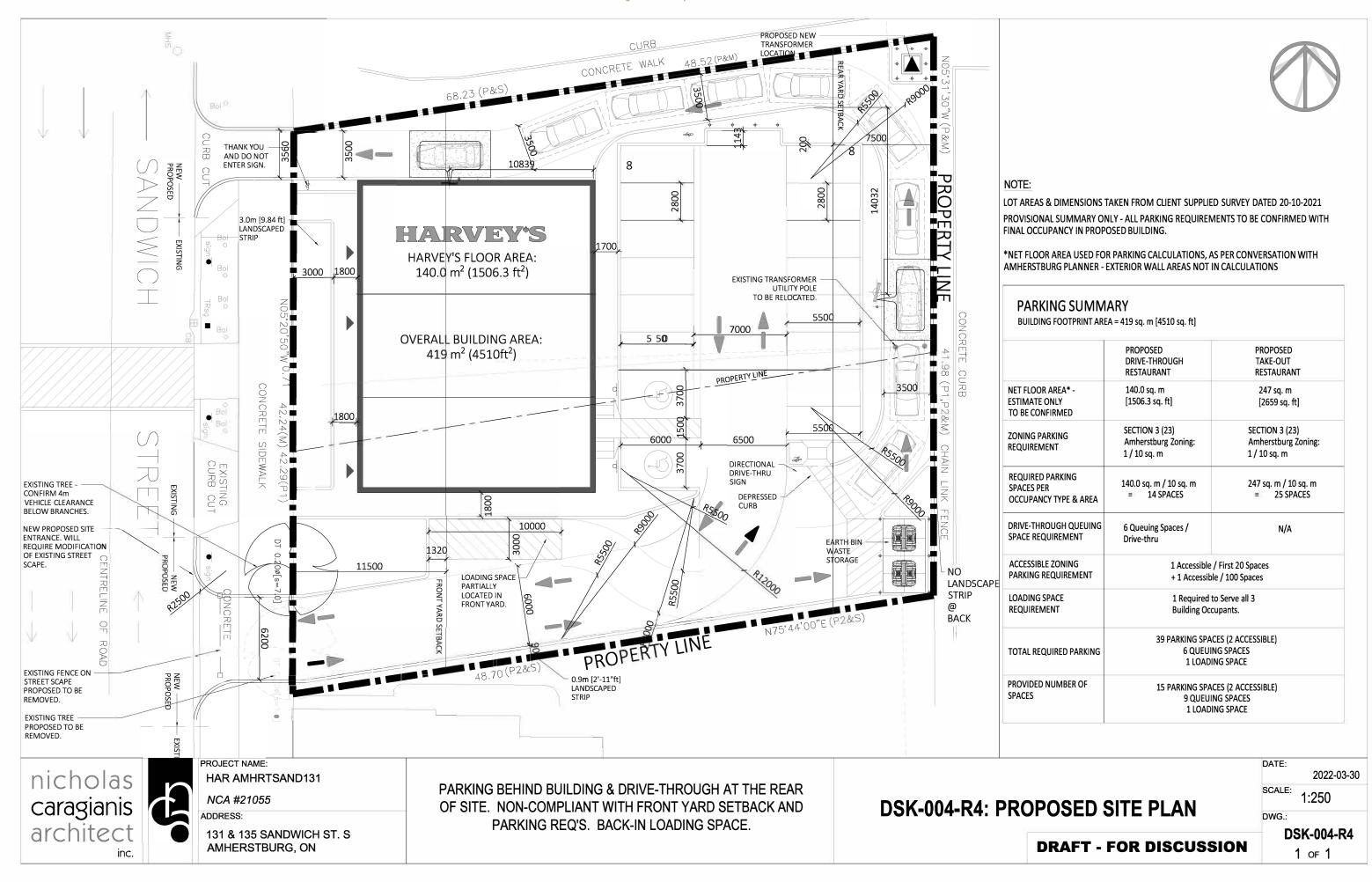
Two site accesses are proposed, the first access being an out-only access located 50 metres south of Fort Street (Site Access #1), and the second being a full-movement access 85 metres south of Fort Street. As customers are anticipated to also park in the existing parking lot north of the subject site, site traffic will also enter and exit the parking lot driveway which is located 40 metres south of Fort Street. These intersection distances are measured intersection centreline to intersection centreline.

The subject site is anticipated to be built-out in one phase with a build-out year in 2023. The scope of this TIA has been confirmed with staff from both Essex County and the Town of Amherstburg in the form of a Terms of Reference (TOR) document and subsequent correspondence which can be seen in Appendix A.

Figure 1 illustrates the Study Area Context. Figure 2 illustrates the concept site plan.







2 Study Area

The Study Area will consist of the following intersections:

- Fort Street and Sandwich Street South
- Parking Lot Driveway and Sandwich Street South
- Signalized Pedestrian Crossing and Sandwich Street South
- North Street / 165 Sandwich Street South Driveway and Sandwich Street South

3 Existing Conditions

3.1 Area Road Network

Sandwich Street South

Sandwich Street South (Essex County Road 20) is a County of Essex major arterial road with a three-lane cross-section and a posted speed limit of 50 km/h. Sidewalks as well as curbs and gutters are present on both sides of Sandwich Street South within the Study Area. The Town of Amherstburg Official Plan reserves a 30.0 metre right-of-way for Sandwich Street South where possible.

Fort Street

Fort Street is a Town of Amherstburg local road with a two-lane cross-section and a posted speed limit of 40 km/h. Sidewalks as well as curbs and gutters are present on both sides of Fort Street within the Study Area. The Town of Amherstburg Official Plan reserves a 20.0 metre right-of-way for local roads.

North Street

North Street is a Town of Amherstburg local road with a two-lane cross-section and an unposted assumed speed limit of 50 km/h. Sidewalks, on-street parking, and curbs and gutters are present on both sides of Fort Street within the Study Area. The Town of Amherstburg Official Plan reserves a 20.0 metre right-of-way for local roads.

3.2 Existing Intersections

Fort Street & Sandwich Street South

The intersection of Fort Street and Sandwich Street South is an unsignalized three-legged intersection with stop control on the westbound approach. The northbound approach consists of a shared through / right-turn lane and the southbound approach consists of an auxiliary left-turn lane and a through lane. The westbound approach has an auxiliary left-turn lane and a right-turn lane. Pedestrian crosswalks are provided on the east leg of the intersection. No turning restrictions were noted at this intersection.





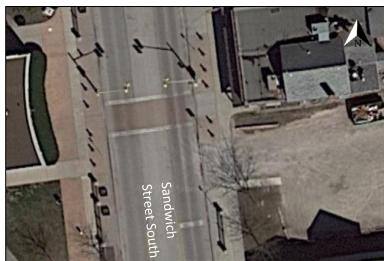
Parking Lot Driveway & Sandwich Street South

The intersection of the parking lot driveway and Sandwich Street South is an unsignalized three-legged intersection with stop-control on the westbound approach. The northbound approach consists of a shared through / right-turn lane and the southbound approach consists of an auxiliary left-turn lane and a through lane. The westbound approach has a shared left-turn lane / right-turn lane. Pedestrian crosswalks are provided on the east leg of the intersection. No turning restrictions were noted at this intersection.



Signalized Pedestrian Crossing & Sandwich Street South

The signalized pedestrian crossing on Sandwich Street South is located adjacent to General Amherst High School as well as the proposed development.



North Street / 165 Sandwich Street South Driveway & Sandwich Street South

The intersection of North Street and Sandwich Street South is an unsignalized four-legged intersection with stop-control on the eastbound and westbound legs. Both the southbound and northbound approach consists of an auxiliary left-turn and shared through / right-turn lane. Both the eastbound and westbound approach have a shared left-turn / right-turn Pedestrian crosswalks are provided on the east and west legs of the intersection. No turning restrictions were noted at this intersection.





3.3 Existing Driveways

Existing driveways along Sandwich Street South within close proximity to the proposed development's accesses are primarily commercial in nature. These driveways may be considered traffic sources and as such may contribute to volume imbalances between Study Area intersections.

3.4 Cycling and Pedestrian Facilities

As described in Section 3.1 above, sidewalks are provided on both sides of Sandwich Street South, Fort Street, and North Street within the Study Area. As no cycling facilities are noted within the Study Area, cyclists will need to share the road with vehicles to facilitate cycling trips.

3.5 Existing Transit

There is no existing transit service noted within the Town of Amherstburg.

3.6 Existing Peak Hour Travel Demand

As shown below, existing Study Area intersection turning movement counts for the weekday AM Peak and weekday PM Peak were collected by Ontario Traffic Inc. Supplementary traffic data was also provided by the Town of Amherstburg. Table 1 summarizes the count locations, data sources, and identified peak hour periods.

AM Peak Hour Location **Count Date Data Source** (PM Peak Hour) Sandwich St S at Fort St Sandwich St S at Parking Lot Thursday February 24, 8:00 - 9:00Ontario Traffic Inc. Driveway 2022 (16:15 - 17:15)Sandwich St S at North St / 165 Sandwich St S Fort St btwn Sandwich St S & **AADT** Balaclava St 2016 Town of Amherstburg North St btwn Sandwich St S **AADT** & Dalhousie St

Table 1: Turning Movement Count Data Dates

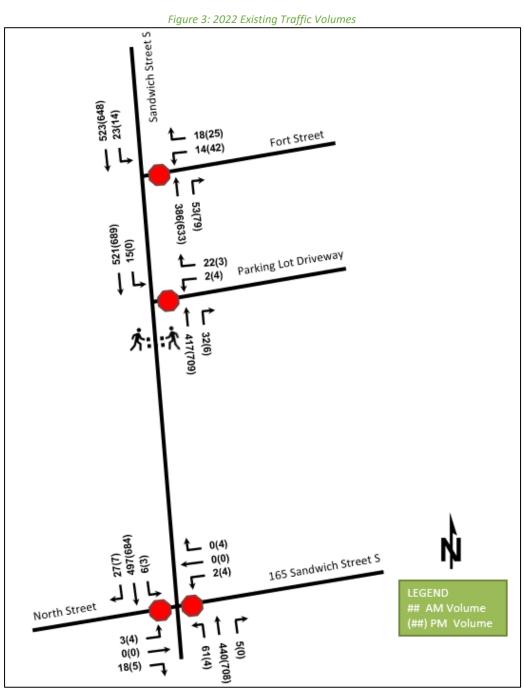
As shown above, the AADT counts have been collected in 2016. To grow these volumes to a consistent analysis horizon of 2022, a compound annual growth rate of 2% has been applied. A growth rate of 2% has been confirmed with Town staff as shown in Appendix A.

As all turning movement count intersection traffic data was collected in 2022, no growth rate is required to be applied to the turning movement counts as they already represent a consistent 2022 horizon. These counts have however been collected during the COVID-19 pandemic. Despite these counts occurring during a time period of minimal COVID-19 restrictions, adjustments to these counts are still required to capture any impact to these volumes. Using the 2022 volumes at the intersection of Sandwich Street South at Fort Street compared to the grown 2016 AADT volumes on Fort Street, an increase factor has been developed. An increase factor has also been calculated using the 2022 volumes at the intersection of Sandwich Street South at North Street / 165 Sandwich Street South compared to the grown 2016 AADT volumes on North Street. In both calculations, the PM peak period volume on Fort Street and on North Street has been calculated by dividing the provided AADT volume by 10, which is a generally accepted approach to determining the PM peak period volume based on AADT volumes. The increase factor developed using the volumes on North Street is 5.26. Given the minimal COVID-19 restrictions in place at the time of traffic data collection, the



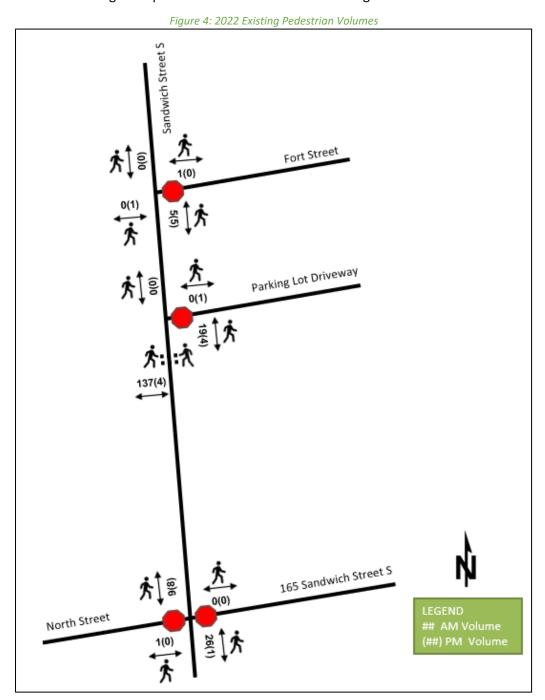
application of an increase factor is likely unnecessary and a relatively low increase factor is expected. As such, the increase factor of 5.26 is not applicable. This result is likely an indication that the generally accepted approach to determining the PM peak period volume based on AADT volumes is not applicable in this instance due to significant traffic generated outside of the AM and PM peak periods as a result of the Austin "Toddy" Jones Park and Bill Wigle park. As such, the calculated adjustment factor of 1.15 has been applied to every movement at all Study Area intersections. The calculations of these adjustment factors can be seen in Appendix B.

Figure 3 illustrates the 2022 existing horizon traffic volumes. Detailed turning movement count data can be found in Appendix C.



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Cycling volumes are noted to be minimal at the Study Area intersections. Pedestrian volumes are not noted to be significant at the Study Area intersections either, however 137 pedestrians have been noted to use the pedestrian crossing during the AM peak period, and four pedestrians are noted to use the pedestrian crossing during the PM peak period. As General Amherst High School is located on the west side of Sandwich Street South adjacent to the pedestrian crossing, these pedestrian volumes are expected as the AM peak hour overlaps with the typical high school start time. The existing 2022 pedestrian volumes are shown in Figure 4 below.





4 Future Background Conditions

4.1 Planned Changes to the Area Transportation Network

The County of Essex's County Wide Active Transportation System (CWATS) indicates that Sandwich Street South within the Study Area is proposed to be a signed bicycle route.

No other changes to the area transportation network are anticipated. This was confirmed through the Terms of Reference submission and subsequent correspondence which can be seen in Appendix A.

4.2 Other Study Area Developments

At the time of this report, no other development applications were available for the adjacent properties. This was confirmed through the Terms of Reference submission and subsequent correspondence which can be seen in Appendix A.

4.3 Background Growth

As discussed in Section 3.6 above, a 2% compound annual growth rate was approved by Town of Amherstburg staff. As such a 2% growth rate has been applied to all Study Area intersection movements to determine the 2023 future background analysis horizon traffic volumes.

4.4 Future Background Traffic Volumes

Combining the background growth rate discussed in Section 4.3 above, the 2022 existing traffic volumes, future background traffic volumes were projected.

Figure 5 illustrates the 2023 future background traffic volumes. All intersection lane configurations have been carried forward from the 2022 existing conditions as there are no anticipated changes for the 2023 horizon.



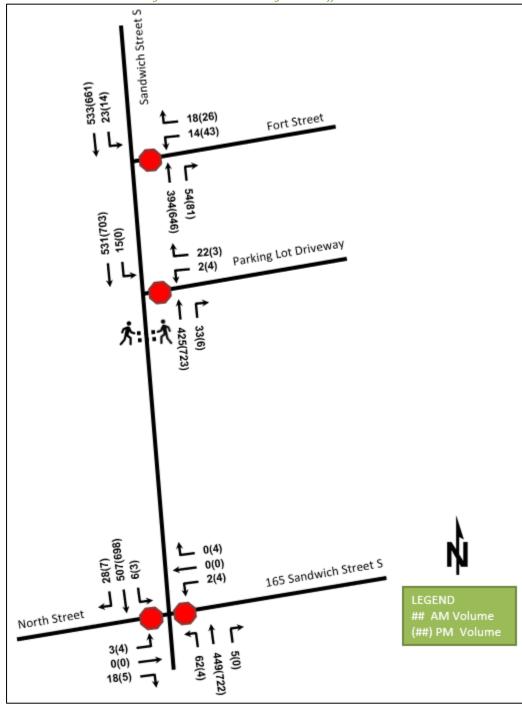


Figure 5: 2023 Future Background Traffic Volumes

5 Demand Forecasting

5.1 Site Trip Generation

The proposed development will include a 140 m^2 drive-through restaurant and two take-out restaurants that total 247 m^2 . All three restaurants will have both indoor seating as well as a take-out counter.



Appropriate trip generation weighted average vehicle trip rates for the restaurants were obtained from the 11th Edition of Institute of Transportation Engineers (ITE) Trip Generation Manual and are summarized in Table 2.

Table 2: ITE Trip Generation Rate

Land Use	Land Use Code	Trip Rates					
Land Ose	Land Ose Code	AM Peak	PM Peak				
Fast Casual Restaurant	ITE 930	T = 1.43(X)	T = 12.55(X)				
Fast-food Restaurant with Drive-Through	ITE 934	T = 44.61(X)	T = 33.03(X)				
Notes: $T = Average \ Vehicle \ Trip \ Ends, \ X = 1000 \ ft^2 \ GFA$							

Based on these rate equations, the total vehicle site trip generation during the weekday AM Peak and weekday PM Peak are summarized in Table 3. It is noted that as the fast-food restaurant with a drive-through is known to be a Harvey's, no AM peak period trip generation has been considered for this land use as Harvey's are not open during the AM peak period.

Table 3: Vehicle Site Trip Generation

Land Use	GFA	AM	Peak (veh/	hr)	PM Peak (veh/hr)		
Land Ose	GFA	In	Out	Total	In	Out	Total
Fast Casual Restaurant	2,659 ft ²	2	2	4	18	15	33
Fast-food Restaurant with Drive-Through	1,506 ft ²	-	-	-	26	24	50
	Total	2	2	4	44	39	83

Where applicable, pass-by trips have been accounted for. The pass-by rate has been selected using the ITE Trip Generation Manual 11th Edition for the fast-food restaurant with drive-through only as no pass-by rates are available for the fast casual restaurant land use. The average pass-by trip percentage for the PM peak period has been identified and is shown in Table 4 below.

Table 4: Land Use Pass-by Rates

	1 4 5 1 7 2 4 1 4 5 5 5 7 4 5 5 5 7 1 4 5 5 5				
Land Use	Pass-by Rate				
Land Ose	AM	PM			
Fast-food Restaurant with Drive- Through	-	55%			

Using the above pass-by rate, the net new vehicle trips have been projected and are shown in Table 5.

Table 5: Net New Vehicle Trips

Land Use	AM Peak	PM Peak (veh/hr)				
Land Ose	In	Out	Total	In	Out	Total
Fast Casual Restaurant	2	2	4	18	15	33
Fast-food Restaurant with Drive- Through	-	-	-	26	24	50
Pass-by	-	-	-	-14	-13	-27
Net New Auto Driver Total	2	2	4	30	26	56

As shown above, four AM, and 56 PM net new peak hour two-way vehicle trips are projected as a result of the proposed development.



5.2 Vehicle Traffic Distribution and Assignment

Traffic distribution was based on the existing volume splits on Sandwich Street South as well as at Study Area intersections and our knowledge of the surrounding area. The resultant distribution is outlined as follows:

- 49% to/from the north along Sandwich Street South;
- 51% to/from the south along Sandwich Street South;

Based on the distribution outlined above, turning movement splits, and access to major transportation infrastructure, the trips generated by the site have been assigned to the Study Area road network.

To assign the pass-by trips to Site Access #1, Site Access #2, and the Parking Lot intersection on Sandwich Street South, a ratio of northbound trips as a portion of all traffic on Sandwich Street South, and southbound trips as a portion of all traffic on Sandwich Street South was developed. It was determined that approximately 51% of the total traffic is northbound and 49% of the total traffic is southbound in the 2023 PM peak period. Using these percents, the traffic has been logically distributed to Site Access #1, Site Access #2, and the Parking Lot driveway intersection.

It has been assumed that 60% of the traffic generated by the Harvey's will use the drive-through, and the remaining 40% will park. As discussed above, parking spaces will be provided on the subject site lands, and additional parking spaces within the adjacent parking lot to the north will likely be used by customers given its proximity to the subject site. In order to reflect this situation, the parking demand for the subject site was determined using the ITE Parking Generation Web Application, 5th Edition. The resulting parking demand can be seen below in Table 6.

Land UseLand Use CodeGFAParking DemandFast Casual RestaurantITE 9302,659 ft²26Fast-food Restaurant
with Drive-ThroughITE 9341,506 ft²13

Table 6: ITE Parking Generation Manual - Parking Demand

As shown above, the parking demand of the proposed development is anticipated to be 39 spaces. It is noted that the Town of Amherstburg Zoning By-law requires 39 parking spaces for the proposed development. As 15 parking spaces (38%) are provided at the subject site, an additional 24 (62%) spaces will likely be used in the adjacent parking lot to the north. As such, 38% of the vehicle trips generated by the proposed development that are anticipated to use a parking space will use Site Access #2, and the remaining 62% of vehicle trips generated by the proposed development that are anticipated to use a parking space will use the Parking Lot Driveway.

Figure 6 illustrates the forecasted site pass-by trips for the 2023 future horizon. Figure 7 illustrates the 2023 new site traffic generated volumes, and Figure 8 illustrates the 2023 net new site generated volumes.



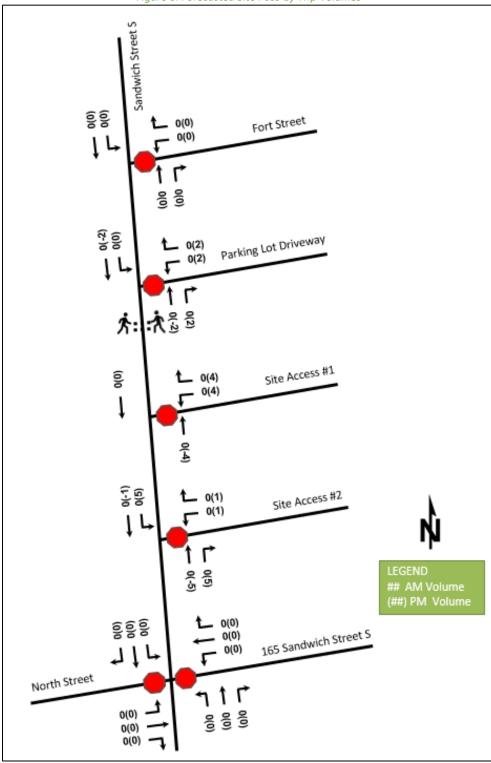


Figure 6: Forecasted Site Pass-by Trip Volumes



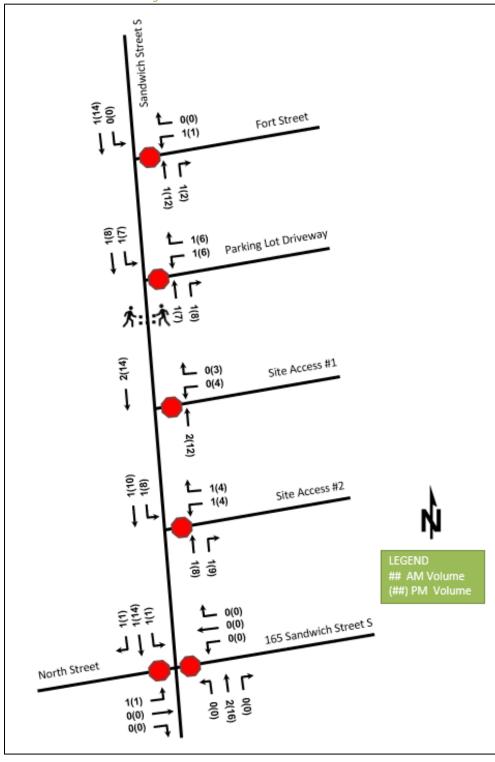


Figure 7: New Site Generated Auto Volumes



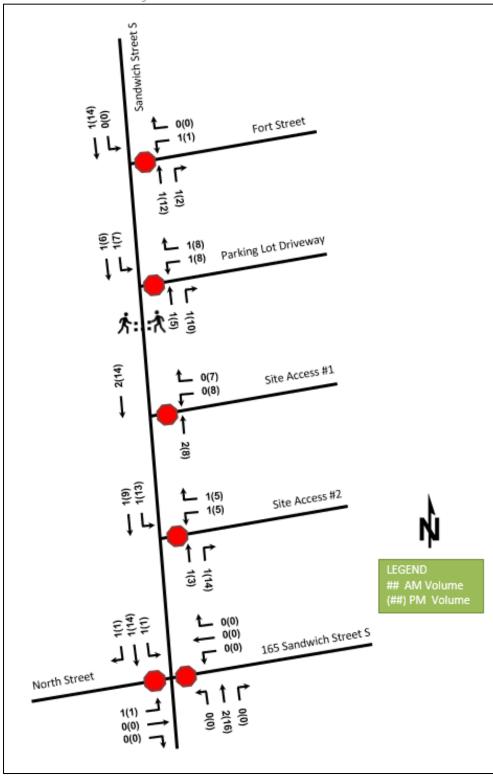


Figure 8: Net-New Site Generated Auto Volumes

5.3 Future Total Travel Demands

The 2023 site generated traffic has been combined with the 2023 future background traffic volumes to estimate the 2023 future total traffic volumes shown in Figure 9. Access configuration details are discussed in Section 6.3.



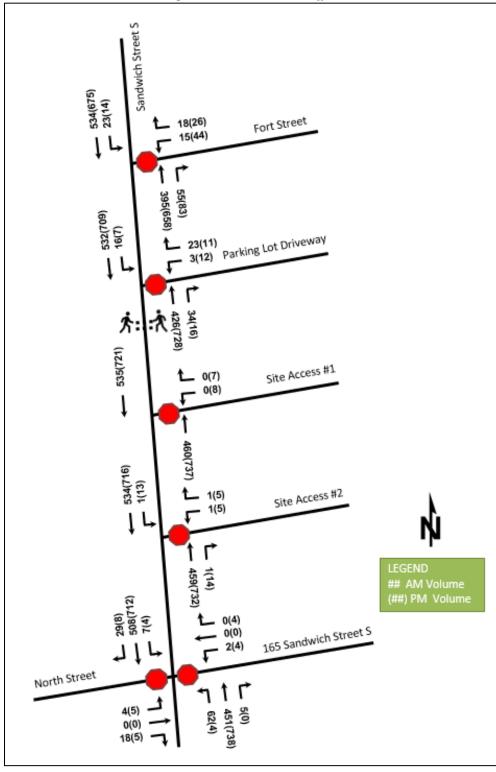


Figure 9: 2023 Future Total Traffic

6 Operational Analysis

To understand the operational characteristics of the Study Area intersections, a Synchro model has been created using Trafficware's Synchro (Version 10).



Heavy Vehicle percentages (HV%) have been calculated for each movement based on the existing turning movement counts for the Study Area intersections and have been applied to both the existing and future analysis horizons. Any HV% calculated to be less than 2% was entered as 2% in Synchro to ensure a conservative analysis. At intersections where no Heavy Vehicle percentage is available, 2% has been used. Heavy Vehicle percentage calculations can be found in Appendix D.

Pedestrian and cyclist volumes were provided for those intersections with turning movement count information collected in 2022 and have been applied to the existing and future conditions analysis. For future intersections where no pedestrian volumes are available, a conservative assumption of 5 pedestrians/h has been used for each intersection leg with pedestrian facilities. For future intersections where no cyclist volumes were provided, a conservative assumption of 5 cyclists/h has been used for each intersection leg where applicable.

Peak Hour Factors (PHF) have been entered for each intersection based on the turning movement counts provided. At future intersections where no 15-minute count data is available, the Peak Hour Factors from the closest adjacent intersection has been used. The Peak Hour Factors used for each intersection are shown in Appendix C and are summarized below in Table 7.

Table 7: Peak Hour Factors

lusta una actio un	Peak Hour Factor						
Intersection	AM	PM					
Sandwich St S at Fort St	0.95	0.97					
Sandwich St S at Parking Lot Driveway	0.96	0.98					
Sandwich St S & North St / 165 Sandwich St S	0.90	0.98					
Sandwich St S at Site Access #1	0.96*	0.98*					
Sandwich St S at Site Access #2	0.96*	0.98*					
*PHF taken from adjacent intersection of Sandwich Street South at the Parking Lot Driveway							

All other parameters have been coded using accepted best practices and default parameters, where applicable.

LOS has been defined using the HCM 2010 definition for LOS at unsignalized intersections (Table 8) and signalized intersections (Table 9).

Table 8: Level of Service Criteria for Unsignalized Intersections

Delay (s)	LOS
≤10	Α
>10 and ≤15	В
>15 and ≤25	С
>25 and ≤35	D
>35 and ≤50	E
>50	F

Table 9: Level of Service Criteria for Signalized Intersections

Average Control Delay (Seconds/Vehicle)	LOS
≤10	A
>10 – 20	В
>20 – 35	С
>35 – 55	D
>55 – 80	E
>80	F



Critical movements and critical intersections have been defined as individual movements with LOS F or a v/c ratio of 1.00 or greater, and intersections with an overall LOS F. Critical movements and critical intersections will be indicated in red below and require mitigation measures.

The 95th percentile queue of each movement was determined using SimTraffic. A minimum of five simulations comprised of a 60-minute simulation and a 15-minute seeding time were used to produce the corresponding queuing reports.

6.1 2022 Existing Operational Analysis

Table 10 summarizes the operational analysis for the 2022 existing conditions in both the AM and PM peak periods. Critical movements, as defined above, have been identified in red where applicable. Synchro and SimTraffic worksheets for the 2022 existing traffic conditions are included in Appendix E.

The Study Area intersections have been designed based on aerial photos and turning lane storage lengths have been rounded to the closest five-metre. All other parameters have been coded using accepted best practices and default parameters where applicable.

The Synchro model has been coded to best reflect the timing information provided by Town of Amherstburg staff regarding the existing signalized pedestrian crossing. As shown in Appendix A, it has been indicated that the signal is callable by pedestrians and has a 30 second pedestrian phase. As no further information was provided, assumptions regarding the signal timing were made. The northbound and southbound signal timing splits as well as the cycle length was optimized, and Amber Clearance and All Red Clearance times were based on methodology provided in OTM Book 12 – Traffic Signals. A walk time of seven seconds was assumed, and a resulting Flash Don't Walk time of 19 seconds was used to allow for a 30 second pedestrian crossing phase.

Table 10: 2022 Existing Intersections Operational Analysis

luda ya a aki a ya	1	AM Peak Hour				PM Peak Hour				
Intersection	Lane	LOS	Delay	V/C	Q (95 th)	LOS	Delay	V/C	Q (95 th)	
	WBL	С	15	0.04	11	С	19	0.15	16	
Canalissiah Ct C O	WBR	В	11	0.03	12	В	14	0.06	14	
Sandwich St S & Fort St	NBT/R	-	-	-	2	-	-	-	4	
	SBL	Α	8	0.02	10	Α	9	0.02	9	
Unsignalized	SBT	-	-	-	18	-	-	-	2	
	Overall	Α	1	-	-	Α	1	-	-	
Complusion Ct C O	WBL/R	В	12	0.05	12	С	16	0.02	7	
Sandwich St S &	NBT/R	-	-	-	1	-	-	-	5	
Parking Lot	SBL	Α	9	0.02	8	Α	0	-	-	
Driveway Signalized	SBT	-	-	-	60*	-	-	-	14	
Signanzea	Overall	Α	<1	-	-	Α	<1	-	-	
	EBL/T/R	С	15	0.06	12	D	26	0.05	10	
Canadantals Ct C O	WBL/T/R	D	33	0.02	4	D	27	0.05	10	
Sandwich St S &	NBL	Α	9	0.07	16	Α	9	0.01	1	
North St / 165 Sandwich St S	NBT/R	-	-	-	5	-	-	-	-	
Unsignalized	SBL	Α	9	0.01	4	Α	9	0.00	3	
Onsignanzea	SBT/R	-	-	-	2	-	-	-	-	
	Overall	Α	1	-	-	Α	<1	-	-	



Interception	Lana	AM Peak Hour				PM Peak Hour			
Intersection	Lane	LOS	Delay	V/C	Q (95 th)	LOS	Delay	V/C	Q (95 th)
Sandwich St S &	NBT	В	13	0.42	67	Α	6	0.44	32
Pedestrian	SBT	В	14	0.48	112*	Α	5	0.42	22
Crossing Signalized	Overall	В	13	-	-	Α	5	-	-
Note:	*Queue ext	*Queue extends beyond available roadway linkage distance.							

The existing Study Area intersections operate satisfactorily during the peak hours with no movements operating with a LOS F and a V/C ratio greater than 1.00. As noted, the southbound through queues on Sandwich Street at the signalized pedestrian crossing, as well as at the Parking Lot Driveway extend beyond the available roadway linkage distance in the AM peak period as a result of the signalized pedestrian crossing. This indicates that the southbound through queues recorded on Sandwich Street South at both the Parking Lot Driveway, and at Front Street are a result of extended vehicle queuing due to the signalized pedestrian crossing. This crossing prioritizes pedestrian safety over vehicle operation on Sandwich Street. As shown above, these movements do not have high v/c ratios, or delays and the queues are expected to clear quickly. As such, no mitigation measures are recommended at the Study Area intersections

6.2 Future Background Conditions

6.2.1 Future Background Traffic Control Warrants

Using Ontario Traffic Manual (OTM) Book 12 Justification 7 methodology for examining traffic control signal warrants, the intersections of Sandwich Street South at Fort Street, Sandwich Street South at the Parking Lot Driveway, and Sandwich Street South at North Street / 165 Sandwich Street South have been evaluated. In the 2023 future background analysis horizon, signalization is not warranted. Traffic control warrant sheets have been included in Appendix F.

6.2.2 Future Background Intersection Design

All Study Area intersections have been analyzed with the same configuration as shown in existing conditions.

6.2.3 Future Background 2023 Conditions

The 2023 future background intersection volumes have been analyzed to allow for a comparison of the future volumes with and without the proposed development.

Table 11 summarizes the operational analysis for the 2023 future background conditions in both the AM and PM peak periods. Critical movements, as defined above, have been identified in red where applicable. The intersections have been analyzed based on the identified signal control and intersection configurations in Section 6.2.1 and Section 6.2.2, respectively. Synchro and SimTraffic worksheets for the 2023 future background traffic conditions are included in Appendix G.

Table 11: 2023 Future Background Conditions Operational Analysis

Intersection	Lana	AM Peak Hour				PM Peak Hour			
intersection	Lane	LOS	Delay	V/C	Q (95 th)	LOS	Delay	V/C	Q (95 th)
	WBL	С	15	0.04	10	С	20	0.15	18
Sandwich St S &	WBR	В	11	0.03	11	В	14	0.06	15
	NBT/R	-	-	-	2	-	-	-	2
Fort St Unsignalized	SBL	Α	8	0.02	9	Α	9	0.02	9
Onsignanzea	SBT	-	-	-	9	-	-	-	7
	Overall	Α	1	-	-	Α	1	-	-



Intersection		AM Peak Hour			PM Peak Hour				
	Lane	LOS	Delay	V/C	Q (95 th)	LOS	Delay	V/C	Q (95 th)
Sandwich St S &	WBL/R	В	12	0.05	13	С	16	0.02	7
	NBT/R	-	-	-	1	-	-	-	6
Parking Lot	SBL	Α	9	0.02	13	Α	0	-	-
Driveway Signalized	SBT	-	-	-	47*	-	-	-	14
	Overall	Α	<1	-	-	Α	<1	-	-
	EBL/T/R	С	15	0.06	12	D	27	0.05	9
	WBL/T/R	D	34	0.02	4	D	28	0.05	10
Sandwich St S &	NBL	Α	9	0.07	17	Α	9	0.01	5
North St / 165	NBT/R	-	-	-	9	-	-	-	-
Sandwich St S <i>Unsignalized</i>	SBL	Α	9	0.01	4	Α	9	0.00	3
	SBT/R	-	-	-	3	-	-	-	-
	Overall	Α	1	-	-	Α	<1	-	-
Sandwich St S &	NBT	В	13	0.42	68	Α	6	0.44	41
Pedestrian Crossing Signalized	SBT	В	14	0.49	103*	Α	5	0.43	44*
	Overall	В	13	-	-	Α	6	-	-
Note:	*Queue extends beyond available roadway linkage distance.								

Generally, the Study Area intersections are shown to operate in a similar manner to the existing conditions with good overall LOS and low delays and no identified critical movements (v/c ratio greater than 0.90 or LOS E or worse). As noted, the southbound through queues on Sandwich Street at the signalized pedestrian crossing, as well as at the Parking Lot Driveway extend beyond the available roadway linkage distance in the AM peak period as a result of the signalized pedestrian crossing. In the PM peak period, the southbound through queue will also extend beyond the available roadway linkage distance. This indicates that the southbound through queues recorded on Sandwich Street South at the Parking Lot Driveway in both the AM and PM peak period, and at Front Street in the AM peak period are a result of extended vehicle queuing due to the signalized pedestrian crossing. This crossing prioritizes pedestrian safety over vehicle operation on Sandwich Street. As shown above, these movements do not have high v/c ratios, or delays and the queues are expected to clear quickly. As such, no mitigation measures are recommended at the Study Area intersections.

6.3 Future Total Conditions

6.3.1 Future Total Traffic Control Warrants

Using the Ontario Traffic Manual (OTM) Book 12 Justification 7 methodology for examining traffic control signal warrants, the intersections of Sandwich Street South at Fort Street, Sandwich Street South at the Parking Lot Driveway, Sandwich Street South at North Street / 165 Sandwich Street South, Sandwich Street South and Access #1, and Sandwich Street South ad Access #2 have been evaluated. Signalization is not warranted at any of these intersections in the 2023 future total analysis horizon. Traffic control warrant sheets have been included in Appendix F.

6.3.2 Future Total Intersection Design

All Study Area intersections have been analyzed with the same configuration as shown in existing conditions.

The intersection of Sandwich Street South and Site Access #1 has been analyzed as an unsignalized intersection with stop-control on the east leg. The intersection consists of a northbound through lane, a southbound through lane, and a shared westbound left-turn / right-turn lane. The east leg functions as the exit to the Harvey's drive-through and does not allow for inbound movements.



The intersection of Sandwich Street South and Site Access #2 has been analyzed as an unsignalized intersection with stop-control on the east leg. The intersection consists of a shared northbound through/right-turn lane, a southbound left-turn lane, a southbound through lane, and a shared westbound left-turn / right-turn lane. The southbound left-turn lane is part of the existing two-way left-turn lane on Sandwich Street South.

6.3.3 Future Total 2023 Conditions

The proposed development's trip generation has been added to the 2023 future background traffic volumes to project the impact of the new traffic on the future road network.

Table 12 summarizes the operational analysis for the 2023 future total conditions in both the AM and PM peak periods. Critical movements, as defined above, have been identified in red where applicable. The intersections have been analyzed based on the identified signal control and intersection configurations in Section 6.3.1 and Section 6.3.2, respectively. Synchro and SimTraffic worksheets for the 2023 future total traffic conditions are included in Appendix H.

Table 12: 2023 Future Total Conditions Operational Analysis

Intersection			AM Pea	k Hour		PM Peak Hour			
	Lane	LOS	Delay	V/C	Q (95 th)	LOS	Delay	V/C	Q (95 th)
Sandwich St S &	WBL	С	15	0.04	14	С	20	0.16	17
	WBR	В	11	0.03	11	В	14	0.06	16
	NBT/R	-	-	-	7	-	-	-	4
Fort St	SBL	Α	8	0.02	9	Α	9	0.02	9
Unsignalized	SBT	-	-	-	8	-	-	-	5
	Overall	Α	1	-	-	Α	1	-	-
	WBL/R	В	12	0.05	12	С	17	0.07	13
Sandwich St S &	NBT/R	-	-	-	5	-	-	-	12
Parking Lot	SBL	Α	9	0.02	10	Α	9	0.01	6
Driveway Signalized	SBT	-	-	-	49*	-	-	-	15
Signalizea	Overall	Α	<1	-	-	Α	<1	-	-
	EBL/T/R	С	16	0.07	11	D	30	0.07	11
	WBL/T/R	D	34	0.02	3	D	29	0.05	9
Sandwich St S &	NBL	Α	9	0.07	15	Α	9	0.01	4
North St / 165	NBT/R	-	-	-	3	-	-	-	-
Sandwich St S Unsignalized	SBL	Α	9	0.01	5	Α	9	0.01	4
	SBT/R	-	-	-	2	-	-	-	-
	Overall	Α	1	-	-	Α	<1	-	-
Sandwich St S &	NBT	В	13	0.43	29*	Α	6	0.45	17*
Pedestrian Crossing Signalized	SBT	В	14	0.50	85*	Α	6	0.44	31*
	Overall	В	13	-	-	Α	6	-	-
Canada dala Ch C O	WBL/R	Α	0	-	-	С	17	0.05	13
Sandwich St S &	NBT	-	-	-	-	-	-	-	4
Access #1	SBT	-	-	-	113*	-	-	-	41*
Unsignalized	Overall	Α	0	-	-	Α	<1	-	-
	WBL/R	В	13	0.00	4	С	17	0.03	9
Sandwich St S & Access #2 Unsignalized	NBT/R	-	-	-	42	-	-	-	18
	SBL	Α	9	0.00	1	Α	9	0.02	6
	SBT	-	-	-	133*	-	-	-	50*
	Overall	Α	0	-	-	Α	<1	-	-
Note:	*Queue ext	ends beyo	nd available	roadway	linkage dista	nce.			



Generally, the Study Area intersections are shown to operate in a similar manner to the 2023 future background conditions with good overall LOS and low delays and no identified critical movements (v/c ratio greater than 0.90 or LOS E or worse). Additionally, the site access intersections are also shown to operate with good overall LOS, and low delays. This indicates that the addition of site traffic from the proposed development will have a minimal impact on the Study Area intersections.

As shown above, the southbound through queues on Sandwich Street at the signalized pedestrian crossing, and the Parking Lot Driveway extend beyond the available roadway linkage distance in the AM peak period as a result of the signalized pedestrian crossing. In the PM peak period, the southbound through queue will also extend beyond the available roadway linkage distance at the signalized pedestrian crossing. This indicates that the southbound through queues recorded on Sandwich Street South at the Parking Lot Driveway in both the AM and PM peak period, and at Fort Street in the AM peak period are a result of extended vehicle queuing due to the signalized pedestrian crossing. The northbound queues on Sandwich Street at the signalized pedestrian crossing also extend beyond the available roadway linkage distance in the AM peak period as a result of the pedestrian crossing. This crossing prioritizes pedestrian safety over vehicle operation on Sandwich Street. As shown above, these movements do not have high v/c ratios, or delays and the queues are expected to clear quickly. As such, no mitigation measures are recommended at the Study Area intersections.

It is also noted that in both the AM and PM peak period, the southbound through queue on Sandwich Street South is shown to extend beyond the available roadway linkage distance at both Site Access #1 and Site Access #2. This is a reflection of Synchro and SimTraffic's limited ability to accurately capture queuing between such closely spaced intersections. In reality these queues are not expected to be realized.

7 Site Plan Review

This section provides an overview of site accesses, site circulation, parking and active mode facilities. The proposed concept Site Plan was previously illustrated in Figure 2.

7.1 Site Circulation

A site circulation assessment was completed using AutoTURN 11.0 to develop turning templates for garbage and loading trucks on site. A Garbage Truck vehicle has been used for the garbage truck circulation analysis and both an HSU and MSU vehicle have been used for the loading truck circulation analysis.

Garbage trucks will enter and exit the site in a forward motion using Site Access #2. Garbage collection will occur on the southeast corner of the subject site and the parking aisle will be used for the trucks to turnaround.

Both HSU and MSU vehicles have been used to test the loading vehicle circulation within the site. Loading vehicles will enter and exit the site in a forward motion using Site Access #2. Loading vehicles will reverse into the loading space and the parking aisle will be used for the trucks to turnaround. HSU vehicles have been used to perform a conservative analysis and are noted to circulate through the site without issue, however some turns may be tight. The MSU analysis is more likely to represent the actual size of the loading vehicle and does not indicate any issues.

Turning template analysis can be found in Appendix I

7.2 Site Access

The proposed development will have two unsignalized site accesses on Sandwich Street South. Two site accesses are proposed, the first access being an out-only access located 50 metres south of Fort Street (Site Access #1), and the second being a full-movement access 85 metres south of Fort Street (Site Access #2). As customers are also anticipated to park at the existing parking lot north of the subject site when required, site traffic will also enter



and exit the parking lot driveway which is located 40 metres south of Fort Street. These intersection distances are measured intersection centreline to intersection centreline.

As discussed above, a signal warrant analysis has been conducted for the 2023 future total horizon using the OTM Book 12 Justification 7 criteria at both proposed site accesses as well as the parking lot driveway. Using this criteria, it was found that a signal is not warranted at any of these intersections.

7.3 Parking Supply

The proposed development will have a total of 15 vehicle parking spaces, nine drive-through queuing spaces, and one loading space. The Town of Amherstburg Zoning By-law indicates the parking requirements for the subject site, and Table 13 below summarizes both the provided and required vehicle parking spaces.

Land Use	Required Parking Rate (spaces/GFA)	Required Parking Spaces	Provided Parking Spaces	Difference	
Restaurant <i>Drive-through</i>	1.00 space/10 m ²	14	15	24	
Restaurant Fast Food & Take-out	1.00 space/10 m ²	25	15	-24	
Total	_	39	_	_	

Table 13: Parking Statistics Summary

As shown above, the proposed development provides 15 vehicle parking spaces which is 24 spaces less than the required number as per the Town of Amherstburg Zoning By-law. These additional 24 required vehicle parking spaces are antipcated to be accommodated in the adjacent parking lot to the north of the subject site. On Thursday, February 24, 2022, a parking count survey of this adjacent parking lot was conducted at the expected peak parking lot occupancy time periods. The maximum number of parked vehicles counted was 56, leaving 97 spaces available. As such, the adjacent parking lot can accommodate the use of 24 of its parking spaces to support the subject site. The parking count data can be found in Appendix J.

Additionally, two accessible parking spaces have been provided on the subject site. As outlined in the Town of Amherstburg Zoning By-law, two accessible parking spaces are required to support the proposed development.

7.4 Queuing Space Supply

Nine queuing spaces are provided to support the Harvey's drive-through which is three spaces in excess of the required six queuing spaces as per the Town of Amherstburg Zoning By-law. This can be seen in Table 14 below.

Table 14: Queuing Space Statistics Summary

Land Use	Required Queuing Spaces	Provided Queuing Spaces	Difference
Restaurant Drive-through	6 spaces	9 spaces	-3 spaces

7.5 Loading Space Supply

The Town of Amherstburg Zoning By-law requires one loading space to support the subject site. As shown in Table 15 below, one loading space is provided and satisfies this requirement.



Table 15: Loading Space Statistics Summary

Land Use	Required Loading Space Rate	Required Loading Spaces	Provided Loading Spaces	Difference
Restaurant Drive-through	1 space to support btwn 250 m ² and 2,500 m ² net floor area	1 space	1 space	0 spaces

It is noted the providing loading space is three metres in width and 10 metres in length. While the loading bay is less than 3.5 metres in with, the trucks are narrower than three metres and can enter and exit the loading space without issues as shown in Appendix I.

7.6 Active Mode Considerations

The proposed development provides active mode facilities and connections within the development as well as connections to Sandwich Street South. Direct connections are provided within the site to the restaurant entrances and parking area and link to the existing sidewalk on the east side of Sandwich Street South. The parking lot to the north is accessible using the sidewalk on the east side of Sandwich Street South. Additionally, as the signalized pedestrian crossing is located along the frontage of the site a direct pedestrian connection is provided to General Amherst High School on the west side of Sandwich Street and allows for greater pedestrian connection to the overall Study Area.

8 Findings and Recommendations

- a) The development, located at 131 & 135 Sandwich Street, includes a 140 m² drive-through restaurant, and 247 m² allocated towards two take-out restaurants.
- b) A total of 15 parking spaces, nine queuing spaces, and one loading space are proposed. 24 additional parking spaces are anticipated to be utilized in the adjacent parking lot to the north.
- c) The proposed development will have two unsignalized accesses located on Sandwich Street South. The first access being an out-only access located 50 metres south of Fort Street (Site Access #1), and the second being a full-movement access 85 metres south of Fort Street.
- d) The full build-out horizon year of 2023 has been analyzed.
- e) No significant planned changes to area transportation network have been noted and no surrounding background developments have been considered.
- f) The proposed development is projected to generate new two-way vehicle volumes of approximately four and 88 veh/h during the weekday morning and afternoon peak hours respectively.
- g) A 2% compound annual growth rate was selected to generate the 2023 future background traffic volumes.
- h) Using the existing 2022 traffic volumes, adjusted for the impact of COVID-19, an operational analysis of existing conditions was undertaken. As no high v/c ratios or high delays were noted, no mitigation measures were recommended.
- i) The 2023 future background operational analysis was similar to the existing analysis as no high v/c ratios, or high delays were noted and therefore no mitigation measures were recommended.
- j) With the addition of site traffic volumes to the Study Area intersections, the intersections operate with minimally worse v/c ratios and higher delays in the 2023 future total horizon. These are minor and do not cause critical movements. Additionally, the site access intersections operate well with no required mitigation measures.
- k) Traffic volumes within the Study Area are relatively low, and as such, signalization is not warranted at any unsignalized intersections at any analysis horizon.



- AutoTURN 11.0 has been used to develop turning templates for garbage trucks and loading trucks within the site with no noted conflicts
- m) On Thursday, February 24, 2022, a parking count survey of the adjacent parking lot north of the subject site was conducted at the expected peak parking lot occupancy time periods. The maximum number of parked vehicles counted was 56, leaving 97 spaces available.
- n) An additional 24 vehicle parking spaces are anticipated to be utilized from the parking lot north of the subject site to fulfill the requirement of the Amherstburg Zoning By-law of 39 vehicle parking spaces.
- o) The subject site will provide nine queuing spaces to support the proposed derive-through which is three spaces in excess of the required six queuing spaces as per the Town of Amherstburg Zoning By-law.
- p) One loading space has been provided to support the subject site which is in accordance with the Town of Amherstburg Zoning By-law.
- q) The proposed development will allow for active transportation through the provision of active mode facilities on-site and through connections to the surrounding Study Area transportation network.

The 131 & 135 Sandwich Street development will have a minor impact on the Study Area road network. The proposed accesses will operate with reasonable LOS and delay on the turning movements into and out of the site. Additionally, through the provision of on-site and off-site facilities, this development will be supportive of active mode transportation. It is recommended that, from a transportation perspective, the proposed development application proceed.

Prepared By:

Reviewed By:

M. B. CROCKFORD HOUSE OF ONTRACTOR

Robin Marinac, E.I.T. 437-242-5183

Polin Marines

Robin.marinac@CGHTransportation.com

Mark Crockford, P. Eng. 905-251-4070

Mark.Crockford@CGHTransportation.com



Appendix A

Terms of Reference and Correspondence



Technical Memorandum

From:	Robin Marinac	Project Number:	2021-139
Cc:	Dino Soltani Nicholas Caragianis		
	Mark Crockford – CGH Transportation		-
To:	Frank Garardo – Town of Amherstburg Jerry Behl – County of Essex	Date:	2022-01-19

Re: 131 & 135 Sandwich Street Transportation Impact Assessment - Terms of Reference

We have been asked to undertake the Transportation Impact Assessment and Parking Justification to support the proposed development located at 131 & 135 Sandwich Street in Amherstburg, Ontario. The development land is currently designated as a General Commercial Zone. The site is proposed to include a 140 m² drive-through restaurant, and 247 m² allocated towards a take-out restaurant land use. A total of 15 parking spaces, nine queuing spaces, and one loading space are proposed. Two site accesses are proposed, the first being a full-movement access 90 metres south of Fort Street, and the second access being an out-only access located 55 metres south of Fort Street. A build-out and occupancy year of 2023 is anticipated.

The proposed site plan can be seen in Attachment 1.

We have prepared the following scope of work for review and endorsement. Please let us know if you have any comments or additions. All data requests are noted in *red* and have also been summarized at the end of the memo.

Transportation Impact Assessment Requirements (TIA):

The study will be in accordance with the *Institute of Transportation Engineers Transportation Impact Analyses* for Site Development.

Study Area:

- An overview of the transportation system existing conditions will be documented (including transit, cycling, pedestrian and automobile modes).
- A summary of existing transportation policies within the Study Area will be identified.
- An overview of the Study Area road network will be provided including the road classification and descriptions of:
 - Fort Street
 - North Street
 - Sandwich Street South
 - The parking lot access north of the site

The following intersections will be included in the Transportation Impact Assessment:

- Fort Street and Sandwich Street South
- North Street and Sandwich Street South
- Parking lot access north of the site
- Signalized Pedestrian Crossing on Sandwich Street along the site frontage (dependent on requested information below)
- All proposed Site Accesses (two accesses on Sandwich Street)

Existing Traffic Data:

- Turning Movement Counts (TMCs) at the Study Area intersections of Fort Street at Sandwich Street South, North Street at Sandwich Street South, and the parking lot access north of the site are requested from County of Essex staff. (Data request)
 - o In the event that TMCs are unavailable at these intersections, existing TMCs will be collected by a third-party consultant.
- AADT/ATR counts collected in 2016 have been indicated to exist on both Fort Street and North Street by Town of Amherstburg staff. Detailed reports of these counts are requested. (Data request)
 - Given the current COVID-19 related restrictions, the collected intersection data will be compared and if needed, factored using a calculated "adjustment factor" based on the AADT/ATR counts discussed above.
- A compound annual growth rate is requested for application to any received TMCs, should they be available, to determine the 2022 existing traffic volumes. (*Data request*)
- Collision data (over the past five years if available) is requested at the four existing intersections that are
 listed above (Fort Street at Sandwich Street South, North Street at Sandwich Street South, the parking
 lot driveway on Sandwich Street north of the subject site, and the signalized pedestrian crossing on
 Sandwich Street), should these be available. (Data request)
- While the signalized pedestrian crossing on Sandwich Street has been indicated above as an intersection
 of interest, the inclusion of this intersection in the TIA depends on the future plans for the General
 Amherst High School and consequently the future need for this pedestrian crossing. Please provide any
 existing pedestrian volumes at this crossing, as well as any anticipated future changes to or removal of
 this signalized crossing. (Data request)

Study Horizon and Peak Periods:

- Base year 2022, followed by a future build-out and occupancy horizon of 2023.
- AM and PM peak hours for all horizons.

Background Growth:

- A compound annual growth rate is requested for application to the 2022 existing intersection volumes to determine the Study Area intersection volumes for the future horizon year of 2023. (*Data request*)
- Surrounding development traffic impact assessments and reports will be used as reference to identify
 additional growth from surrounding developments in the area. Any relevant reports are requested.
 (Data request)
- Please provide additional information on the future plans for the General Amherst High School. (Data request)



Changes to Area Transportation Network

- The County of Essex's County Wide Active Transportation System (CWATS) indicates that Sandwich Street South within the Study Area is proposed to be a signed bicycle route.
- No other changes to the Study Area transportation network are noted. Please provide information on any future changes to the Study Area Transportation network to be considered. (*Data request*)

Development Site Traffic:

- Trip generation: ITE Trip Generation Manual 11th Edition.
- Existing Modal Split: If applicable, please provide modal splits to be used. (Data request)
- Trip distribution and assignment of auto trips: Surrounding area characteristics.

Traffic Analysis:

- Traffic analysis to be performed using Synchro 10 on Study Area network intersections to determine the LOS, delay, V / C ratio and the 95th percentile queues for overall intersections as well as individual movements using Highway Capacity Manual 2010 (HCM) methodology.
 - Heavy Vehicle %, pedestrian volumes, and cyclist volumes will be taken from the collected TMC data. Where information is not available, a pedestrian volume of 5 pedestrians/hour, a cyclist volume of 5 cyclists/hour, and a Heavy Vehicle % of 2% will be used.
 - Other Synchro inputs will be based on site observations and Synchro default parameters.
- A qualitative transit, cycling, and pedestrian analysis including consideration of any planned improvements.

Site Review:

- Parking space analysis and justification will be completed using the Town of Amherstburg's Zoning Bylaw (2019) with consideration given to on-street parking as well as the use of two adjacent municipal parking lots.
- Turning templates within the site will be completed to evaluate on-site circulation.
- Qualitative access location analysis where necessary.

Recommendations:

 Any recommended offsite and onsite improvements or mitigation measures, which may include turn lane requirements, pedestrian / cycling / transit amenities, TDM measures, construction impacts, safety measures etc.

The following is a list of requested information, some of which has been indicated in *red* above, that we are requesting to inform the TIS:

- Any other guidelines you would like us to consider.
- TMC and AADT/ATR data as specified above.
- Historical collision data as specified above.
- Growth rates to be applied to Study Area intersections.
- Future plans for the General Amherst High School.
- Future plans for the signalized pedestrian crossing adjacent to the subject site.



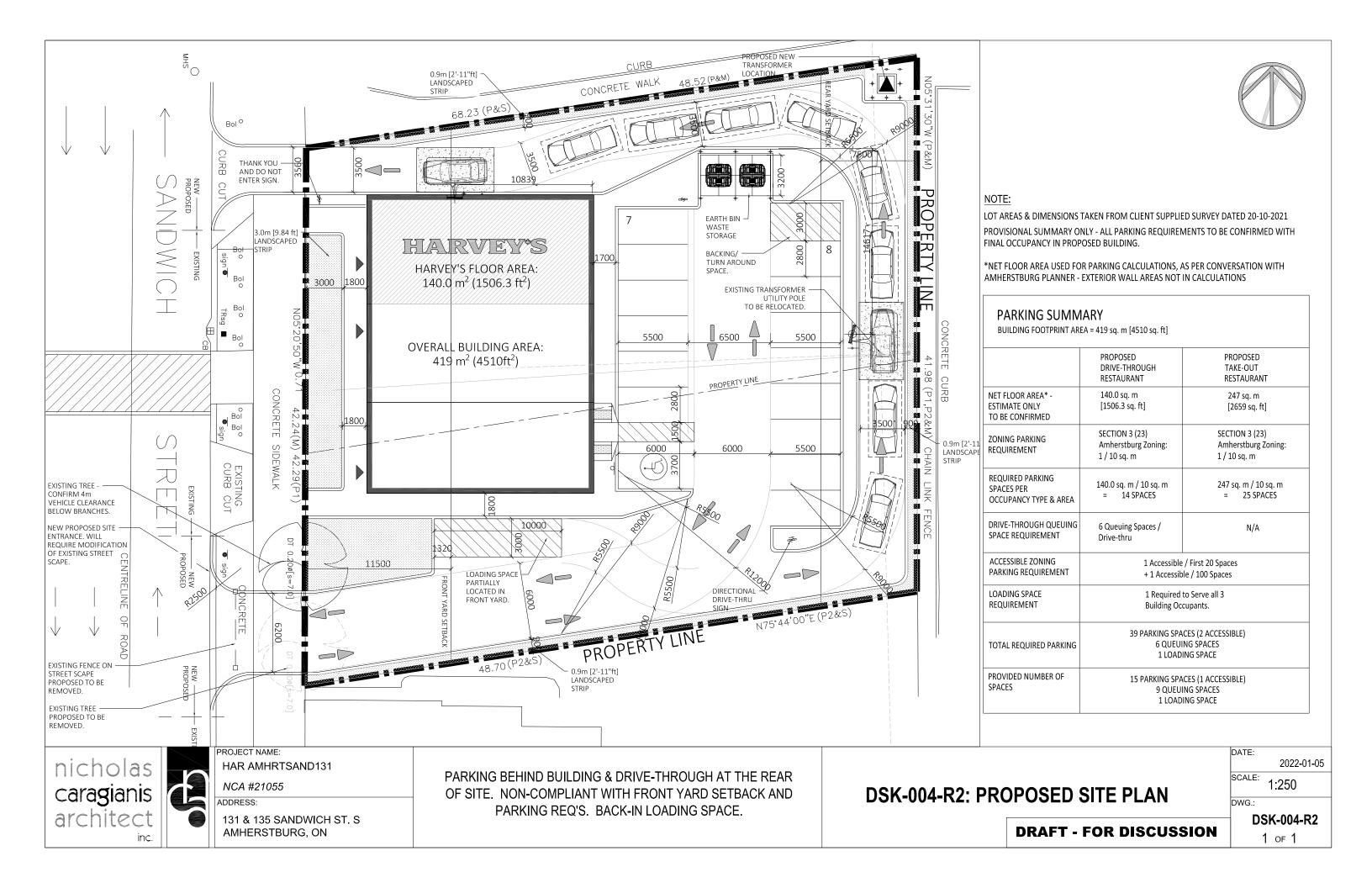
- Any relevant developments that may influence the background growth within the proposed Study Area.
- Suggested modal splits if required.
- Specific changes to the Study Area Road network that you would like us to consider.



Attachment 1

Proposed Site Plan





Robin Marinac

From: Kristoffer Balallo < KBalallo@countyofessex.ca>

Sent: February 2, 2022 2:46 PM

To: Robin Marinac

Cc: Jerry Behl; Ray Sayyadi

Subject: RE: 131 & 135 Sandwich Street TIA Terms of Reference Scoping Document

Good afternoon Robin,

From a County of Essex perspective, we have no comments pertaining to the proposal. The location of the proposed is outside of the County's jurisdiction.

Kind Regards



Kristoffer Balallo

Engineering Technologist County of Essex 360 Fairview Ave. W. Suite 315 | Essex, ON | N8M 1Y6

P: 519-776-6441 ext. 1564

F: 519-776-4455 TTY: 1-877-624-4832

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From: Robin Marinac < robin.marinac@cghtransportation.com >

Sent: January 19, 2022 11:30 AM

To: Frank Garardo <fgarardo@amherstburg.ca>; Jerry Behl <JBehl@countyofessex.ca>

Cc: Mark Crockford <mark.crockford@cghtransportation.com>; dino.soltani80@gmail.com; Nicholas Caragianis at

Nicholas Caragianis Architect Inc. <nicholas@ncarchitect.ca>

Subject: 131 & 135 Sandwich Street TIA Terms of Reference Scoping Document

CAUTION: This email originated from outside of the organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.

Hi Frank and Jerry,

Please find attached our 131 & 135 Sandwich Street South Terms of Reference (TOR) for your review. Please let us know if you have any comments or questions as we would like to ensure that our TOR reflects the appropriate scope of work to support the proposed development.

Kind regards, Robin Marinac



Robin Marinac

From: Todd Hewitt <thewitt@amherstburg.ca>

Sent: February 11, 2022 2:32 PM **To:** Robin Marinac; Frank Garardo

Cc: Mark Crockford

Subject: RE: 131 & 135 Sandwich Street TIA Terms of Reference Scoping Document

Attachments: 21_Balaclava St S & Fort St.xlsx; 21_Balaclava St S & Fort St.pdf

Robin,

See answers below in red

Todd

Todd Hewitt

Manager of Engineering and Operations
Town of Amherstburg
512 Sandwich St South, Amherstburg, ON, N9V 3R2

Tel: 519-736-3664 ext 2313 Fax: 519-736-7080 TTY: 519-736-9860





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From: Robin Marinac < robin.marinac@cghtransportation.com>

Sent: February 3, 2022 4:00 PM

To: Frank Garardo <fgarardo@amherstburg.ca>; Todd Hewitt <thewitt@amherstburg.ca>

Cc: Mark Crockford <mark.crockford@cghtransportation.com>

Subject: RE: 131 & 135 Sandwich Street TIA Terms of Reference Scoping Document

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Hi Frank,

Thank you for the feedback and approval on our Terms of Reference. I've summarized our remaining information and data requests below and included Todd on this email for his assistance. Our requests are as follows:

• Unless otherwise noted, we will assume a 2% compound annual growth rate at our Study Area intersections. As no planned background developments were indicated to be considered, 2% is deemed an appropriate growth

- rate to capture any increase in background traffic. Please clarify that this is a correct assumption or alternatively provide a growth rate to be used to develop future background intersection volumes. 2% is acceptable
- ATR/AADT counts on Fort Street and North Street were provided from 2016. It was indicated that ATR/AADT counts are also available at these locations from 2021, however they are skewed because of the pandemic. Could you please provide these 2021 counts, and if available the raw data as well. Balaclava and Fort intersection data attached.

Kind regards, Robin Marinac



Robin Marinac, EIT CGH Transportation Inc.

P: 437-242-5183

E: robin.marinac@cghtransportation.com

From: Frank Garardo <fgarardo@amherstburg.ca>

Sent: February 2, 2022 1:25 PM

To: Robin Marinac < robin.marinac@cghtransportation.com >; jbehl@countyofessex.ca

Cc: Mark Crockford < mark.crockford@cghtransportation.com >

Subject: RE: 131 & 135 Sandwich Street TIA Terms of Reference Scoping Document

Good afternoon,

The terms of reference are fine. Please send any data request in an email format to Todd Hewitt from the Public works department.

Todd Hewitt

Manager of Engineering and Operations
Town of Amherstburg
512 Sandwich St South, Amherstburg, ON, N9V 3R2

Tel: 519-736-3664 ext 2313 Fax: 519-736-7080 TTY: 519-736-9860

Frank Garardo

Manager, Planning Services
Town of Amherstburg
3295 Meloche Rd., Amherstburg, ON, N9V 2Y8

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From: Robin Marinac < robin.marinac@cghtransportation.com>

Sent: February 1, 2022 2:34 PM

To: Frank Garardo < fgarardo@amherstburg.ca >; jbehl@countyofessex.ca

Cc: Mark Crockford <mark.crockford@cghtransportation.com>

Subject: RE: 131 & 135 Sandwich Street TIA Terms of Reference Scoping Document

[EXTERNAL EMAIL] DO NOT CLICK links or attachments unless you recognize the sender and know the content is safe.

Hi Frank and Jerry,

I hope you are both doing well. As you can see below, our client is pushing for a quick turnaround on our report. As our Terms of Reference scoping document requests your feedback, additional information, and approval regarding our scope of work we are eager to get your responses as quickly as possible. With this in mind, can you please provide us with an idea of when we will hear back from you regarding our Terms of Reference document?

Kind regards, Robin Marinac



Robin Marinac, EIT CGH Transportation Inc.

P: 437-242-5183

E: robin.marinac@cghtransportation.com

From: Nicholas Caragianis at Nicholas Caragianis Architect Inc. < nicholas@ncarchitect.ca >

Sent: February 1, 2022 10:06 AM

To: Robin Marinac < robin.marinac@cghtransportation.com; Frank Garardo fgarardo@amherstburg.ca; jbehl@countyofessex.ca

Cc: Mark Crockford < <u>mark.crockford@cghtransportation.com</u>>; <u>dino.soltani80@gmail.com</u>; Jessica Penn at Nicholas Caragianis Architect Inc. < <u>jpenn@ncarchitect.ca</u>>; Scott Allen < <u>sallen@mhbcplan.com</u>>; <u>nvanoyen@mhbcplan.com</u>

Subject: 131 & 135 Sandwich Street TIA Terms of Reference Scoping Document

Robin – please follow up with the Municipality as the client wishes to submit the applications ASAP.

Nicholas Caragianis

BArch, MBAIC, CAA, GAQ, NSAA email recholos@incorchitectical cel - 613 850 0389



137 Pamela Street, Ottowa, ON K15 3K9 telephone 613 937 6601 for 613 937 6989 www.ncarchitect.ca

for the confidential of a change and yielder and the mail terretarily conserved date, and an effect of an india or against and a conserved formation and an artist applied or deliver as the conserved for information and are or accepted you have recovered to end or area or after the conserved for the story or story or conserved for a Or to prompted finely that good with its conserved each of the served makes be. From: Robin Marinac [mailto:robin.marinac@cghtransportation.com]

Sent: January 19, 2022 11:30 AM

To: Frank Garardo <<u>fgarardo@amherstburg.ca</u>>; <u>jbehl@countyofessex.ca</u>

Cc: Mark Crockford <mark.crockford@cghtransportation.com>; dino.soltani80@gmail.com; Nicholas Caragianis at

Nicholas Caragianis Architect Inc. < nicholas@ncarchitect.ca>

Subject: 131 & 135 Sandwich Street TIA Terms of Reference Scoping Document

Hi Frank and Jerry,

Please find attached our 131 & 135 Sandwich Street South Terms of Reference (TOR) for your review. Please let us know if you have any comments or questions as we would like to ensure that our TOR reflects the appropriate scope of work to support the proposed development.

Kind regards, Robin Marinac



Robin Marinac

From: Todd Hewitt <thewitt@amherstburg.ca>

Sent: January 19, 2022 8:50 AM

To: Robin Marinac
Cc: Frank Garardo

Subject: RE: Traffic Data Request - Sandwich Street TIS and Parking Study

Robin,

I would suggest that the your firm arrange to have the traffic counts completed directly.

Todd

From: Robin Marinac < robin.marinac@cghtransportation.com>

Sent: January 19, 2022 8:22 AM

To: Todd Hewitt <thewitt@amherstburg.ca> **Cc:** Frank Garardo <fgarardo@amherstburg.ca>

Subject: RE: Traffic Data Request - Sandwich Street TIS and Parking Study

[EXTERNAL EMAIL] DO NOT CLICK links or attachments unless you recognize the sender and know the content is safe.

Hi Todd,

Thank you for letting me know. Do you know who we could contact to get that original data collection information?

Kind regards, Robin Marinac



Robin Marinac, EIT CGH Transportation Inc.

P: 437-242-5183

E: robin.marinac@cghtransportation.com

From: Todd Hewitt < thewitt@amherstburg.ca>

Sent: January 18, 2022 10:50 AM

To: Robin Marinac <robin.marinac@cghtransportation.com>

Cc: Frank Garardo <fgarardo@amherstburg.ca>

Subject: RE: Traffic Data Request - Sandwich Street TIS and Parking Study

Robin,

We do not have that type of data.

Todd

From: Robin Marinac < robin.marinac@cghtransportation.com >

Sent: January 18, 2022 9:41 AM

To: Todd Hewitt < thewitt@amherstburg.ca > **Cc:** Frank Garardo < fgarardo@amherstburg.ca >

Subject: RE: Traffic Data Request - Sandwich Street TIS and Parking Study

[EXTERNAL EMAIL] DO NOT CLICK links or attachments unless you recognize the sender and know the content is safe.

Hi Todd,

Thank you for that information. As we begin to get further into our study, I was wondering if you could provide the actual count data for the 2016 AADT/ATR counts at Fort Street and North Street discussed below. I believe this data is typically shown in 15-minute increments.

Kind regards, Robin Marinac



Robin Marinac, EIT CGH Transportation Inc.

P: 437-242-5183

E: robin.marinac@cghtransportation.com

From: Todd Hewitt <thewitt@amherstburg.ca>

Sent: January 17, 2022 12:54 PM

To: Robin Marinac < robin.marinac@cghtransportation.com >

Cc: Frank Garardo < fgarardo@amherstburg.ca >

Subject: RE: Traffic Data Request - Sandwich Street TIS and Parking Study

The pedestrian crossing is strictly on demand. I believe it is a 30 second crossing.

Todd

From: Robin Marinac < robin.marinac@cghtransportation.com >

Sent: January 17, 2022 11:57 AM

To: Todd Hewitt < thewitt@amherstburg.ca Cc: Frank Garardo qrange-qamherstburg.ca samherstburg.ca samherstburg.c

Subject: RE: Traffic Data Request - Sandwich Street TIS and Parking Study

[EXTERNAL EMAIL] DO NOT CLICK links or attachments unless you recognize the sender and know the content is safe.

Hi Todd,

Thank you for your responses below. My request for a STP refers to a Signal Timing Plan. Sorry for the confusion.

Kind regards, Robin Marinac



Robin Marinac, EIT

CGH Transportation Inc.

P: 437-242-5183

E: robin.marinac@cghtransportation.com

From: Todd Hewitt <thewitt@amherstburg.ca>

Sent: January 14, 2022 2:46 PM

To: Robin Marinac <robin.marinac@cghtransportation.com>

Cc: Frank Garardo < fgarardo@amherstburg.ca >

Subject: RE: Traffic Data Request - Sandwich Street TIS and Parking Study

Robin,

See answers below in RED

Todd

Todd Hewitt

Manager of Engineering and Operations Town of Amherstburg 512 Sandwich St South, Amherstburg, ON, N9V 3R2

Tel: 519-736-3664 ext 2313 Fax: 519-736-7080 TTY: 519-736-9860





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From: Frank Garardo

Sent: January 14, 2022 1:44 PM

To: Todd Hewitt <thewitt@amherstburg.ca>

Subject: FW: Traffic Data Request - Sandwich Street TIS and Parking Study

Frank Garardo

Manager, Planning Services
Town of Amherstburg

3295 Meloche Rd., Amherstburg, ON, N9V 2Y8

Tel: Fax: TTY: 519-736-9860





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From: Robin Marinac < robin.marinac@cghtransportation.com >

Sent: January 14, 2022 9:10 AM

To: Frank Garardo <fgarardo@amherstburg.ca>

Subject: RE: Traffic Data Request - Sandwich Street TIS and Parking Study

[EXTERNAL EMAIL] DO NOT CLICK links or attachments unless you recognize the sender and know the content is safe.

Hi Frank,

I realized I hadn't included the figure that was referenced. I've included it below:



Kind regards, Robin Marinac

Robin Marinac, EIT CGH Transportation Inc.



From: Robin Marinac

Sent: January 13, 2022 11:49 AM To: fgarardo@amherstburg.ca

Subject: Traffic Data Request - Sandwich Street TIS and Parking Study

Hi Frank,

I am a traffic consultant looking to obtain traffic and transportation data for use in a TIS for 131 Sandwich Street – a site I believe you are familiar with. I apologize if you are not the correct person to contact for this information and ask that you please forward my request to the appropriate individual.

Could you please provide the following data if available:

- TMC at Sandwich Street South and Fort Street We do not track this
- TMC at North Street and Sandwich Street South We do not track this
- TMC at Sandwich Street South and the parking lot access shown in the figure below We do not track this
- STP for the Signalized Pedestrian Crossing on Sandwich Street, approximately 75 metres south of Fort Street Unfamiliar with STP terminology
- Any ATR/AADT counts along Sandwich Street between Maple Avenue and Rankin Avenue Refer to County of Essex mapping data for Sandwich Street information (if any)
- Any ATR/AADT counts along Fort Street between Sandwich Street and Balaclava Street 1400 2016 (2021 counts skewed by pandemic)
- Any ATR/AADT counts along North Street between Sandwich Street and Dalhousie Street 700 2016 (2021 counts skewed by pandemic)
- Any collision data (preferably over the past 5 years) at the intersections of Sandwich Street at Fort Street and Sandwich Street and North Street. Do not track
- Any collision data (preferably over the past 5 years) along Sandwich Street between Maple Avenue and Rankin Avenue Do not track
- Any collision data (preferably over the past 5 years) along Fort Street between Sandwich Street and Balaclava Street Do not track
- Any collision data (preferably over the past 5 years) along North Street between Sandwich Street and Dalhousie
 Street Do not track

Thank you very much in advance for your help.

Kind regards, Robin Marinac



Robin Marinac, EIT CGH Transportation Inc. P: 437-242-5183

E: robin.marinac@cghtransportation.com

Appendix B

COVID-19 Adjustment Factor Calculations

					Sandwich St	S & Fort St								
NBL	NBL NBT NBR WBL WBT WBR SBL SBT SBR EBL EBT EBR													
0	335	46	12	0	15	20	454	0	0	0	0			
0	550	68	36	0	21	12	563	0	0	0	0			

AAD ⁻	Γ Fort St btwn Sa	andwich St & Ba	alaclava St
2016	1400	PM	140
2022	1577	PIVI	158

	2022	East of Sandw	ich St
	AADT	TMC	Increase Factor
AM	-	-	-
PM	158	137	1.15

				Sandwic	h St S & North	n St/165 Sandw	vich St								
NBL	NBL NBT NBR WBL WBT WBR SBL SBT SBR EBL EBT EBR														
53	382	4	1	0	0	5	432	23	2	0	15				
3	615	0	3	0	3	2	594	6	3	0	4				

AAD ⁻	Γ Fort St btwn Sa	andwich St & Ba	alaclava St
2016	700	PM	70
2022	789	PIVI	79

	2022	West of Sandw	rich St
	AADT	TMC	Increase Factor
AM	=	-	-
PM	79	15	5.26

Appendix C

Traffic Data



Project #22-044 - CGH Transportation

Intersection Count Report

Intersection: Sandwich St S & Fort St

Municipality: Amherstburg

Count Date: Feb 24, 2022

Site Code: 2204400001

Count Categories: Cars, Trucks, Buses, Bicycles, Pedestrians

Count Period: 07:00-09:00, 16:00-18:00

Weather: Clear

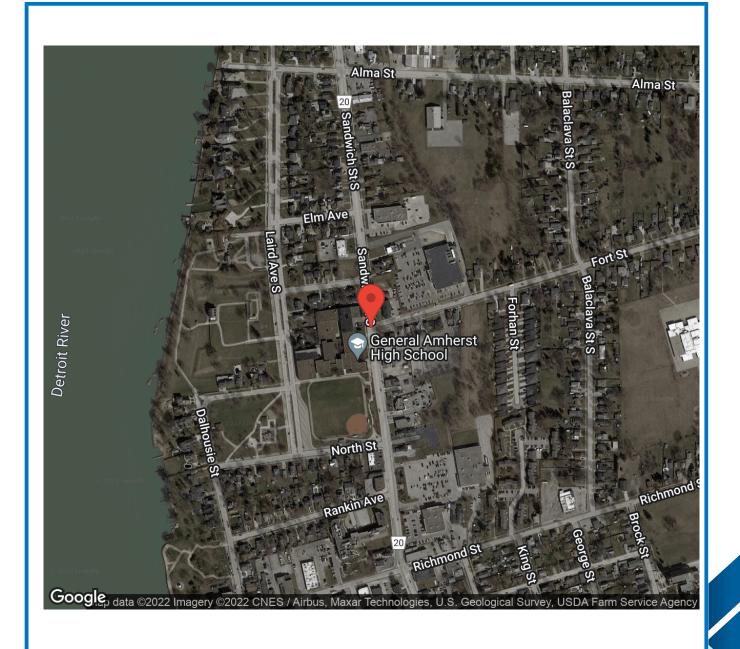


Traffic Count Map

Intersection: Sandwich St S & Fort St

Site Code: 2204400001 Municipality: Amherstburg

Count Date: Feb 24, 2022





Traffic Count Summary

Intersection: Sandwich St S & Fort St

Site Code: 2204400001

Municipality: Amherstburg

Count Date: Feb 24, 2022

Sandwich St S - Traffic Summary

		North	Appr	oach T	otals			South	Appr	oach T	otals		
	Inc	cludes Ca	ars, Truc	ks, Buses	s, Bicycle	s	Inc	cludes Ca	rs, Truc	ks, Buses	s, Bicycle	S	
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	7	244	0	0	251	1	0	310	18	0	328	0	579
08:00 - 09:00						0	335	46	0	381	0	855	
					В	REAK							
16:00 - 17:00	14	580	0	0	594	1	0	515	77	0	592	1	1186
17:00 - 18:00	8	497	0	0	505	0	0	449	64	0	513	0	1018
GRAND TOTAL	49	1775	0	0	1824	3	0	1609	205	0	1814	1	3638



Traffic Count Summary

Intersection: Sandwich St S & Fort St

Site Code: 2204400001

Municipality: Amherstburg

Count Date: Feb 24, 2022

Fort St - Traffic Summary

		East /	Appro	ach To	tals			West	Appro	oach To	otals		
	Inc	ludes Ca	rs, Truc	ks, Buses	, Bicycle	:S	Inc	ludes Ca	rs, Truc	ks, Buses	, Bicycle	es.	
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	7	0	7	0	14	1	0	0	0	0	0	0	14
08:00 - 09:00	12	0	15	0	27	5	0	0	0	0	0	0	27
					В	REAK							
16:00 - 17:00	40	0	22	0	62	6	0	0	0	0	0	0	62
17:00 - 18:00	31	0	9	0	40	2	0	0	0	0	0	0	40
GRAND TOTAL	90	0	53	0	143	14	0	0	0	0	0	0	143



Intersection: Sandwich St S & Fort St

Site Code: 2204400001

Municipality: Amherstburg

Count Date: Feb 24, 2022

North Approach - Sandwich St S

			Cars				T	rucks				I	Buses				В	icycles			Total
Start Time	4	1	•	1	Total	4	1	•	J.	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	2	37	0	0	39	0	2	0	0	2	0	1	0	0	1	0	0	0	0	0	0
07:15	1	45	0	0	46	0	2	0	0	2	0	2	0	0	2	0	0	0	0	0	0
07:30	1	62	0	0	63	0	4	0	0	4	0	5	0	0	5	0	0	0	0	0	1
07:45	3	79	0	0	82	0	2	0	0	2	0	3	0	0	3	0	0	0	0	0	0
08:00	11	95	0	0	106	0	1	0	0	1	0	3	0	0	3	0	0	0	0	0	0
08:15	7	117	0	0	124	0	1	0	0	1	0	2	0	0	2	0	0	0	0	0	0
08:30	1	99	0	0	100	0	4	0	0	4	0	0	0	0	0	0	0	0	0	0	0
08:45	1	124	0	0	125	0	6	0	0	6	0	2	0	0	2	0	0	0	0	0	1
SUBTOTAL	27	658	0	0	685	0	22	0	0	22	0	18	0	0	18	0	0	0	0	0	2



Intersection: Sandwich St S & Fort St

Site Code: 2204400001

Municipality: Amherstburg

Count Date: Feb 24, 2022

North Approach - Sandwich St S

			Cars				T	rucks				ı	Buses				Bi	cycles			Total
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
16:00	3	142	0	0	145	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	1
16:15	5	146	0	0	151	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	C
16:30	1	147	0	0	148	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	5	140	0	0	145	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	C
17:00	1	127	0	0	128	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	2	133	0	0	135	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
17:30	3	115	0	0	118	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
17:45	2	119	0	0	121	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0
SUBTOTAL	22	1069	0	0	1091	0	5	0	0	5	0	3	0	0	3	0	0	0	0	0	1
GRAND TOTAL	49	1727	0	0	1776	0	27	0	0	27	0	21	0	0	21	0	0	0	0	0	3



Intersection: Sandwich St S & Fort St

Site Code: 2204400001

Municipality: Amherstburg

Count Date: Feb 24, 2022

South Approach - Sandwich St S

			Cars				T	rucks				В	uses				Bi	cycles			Total
Start Time	4	1	•	1	Total	4	1	•	J.	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	0	63	6	0	69	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0
07:15	0	74	1	0	75	0	3	1	0	4	0	3	0	0	3	0	0	0	0	0	0
07:30	0	84	4	0	88	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0
07:45	0	74	5	0	79	0	3	1	0	4	0	1	0	0	1	0	0	0	0	0	0
08:00	0	101	9	0	110	0	3	0	0	3	0	3	0	0	3	0	0	0	0	0	0
08:15	0	73	20	0	93	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0
08:30	0	78	6	0	84	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0
08:45	0	72	10	0	82	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	0	619	61	0	680	0	17	3	0	20	0	9	0	0	9	0	0	0	0	0	0



Intersection: Sandwich St S & Fort St

Site Code: 2204400001

Municipality: Amherstburg

Count Date: Feb 24, 2022

South Approach - Sandwich St S

			Cars				Ti	rucks				В	Buses				Bi	cycles			Total
Start Time	4	1	•	1	Total	4	1	•	J.	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
16:00	0	111	21	0	132	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0
16:15	0	123	17	0	140	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
16:30	0	138	17	0	155	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	1
16:45	0	136	22	0	158	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0
17:00	0	145	12	0	157	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0
17:15	0	115	19	0	134	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	0	95	17	0	112	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45	0	92	15	0	107	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	0	955	140	0	1095	0	8	1	0	9	0	1	0	0	1	0	0	0	0	0	1
GRAND TOTAL	0	1574	201	0	1775	0	25	4	0	29	0	10	0	0	10	0	0	0	0	0	1



Intersection: Sandwich St S & Fort St

Site Code: 2204400001

Municipality: Amherstburg

Count Date: Feb 24, 2022

East Approach - Fort St

			Cars			Trucks					Buses					Bicycles					Total
Start Time	4	1		J.	Total	4	1	•	J.	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	0	0	3	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15	3	0	1	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
07:30	1	0	1	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45	3	0	2	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	1	0	5	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
08:15	5	0	5	0	10	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
08:30	5	0	2	0	7	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45	0	0	3	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
SUBTOTAL	18	0	22	0	40	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	6



Intersection: Sandwich St S & Fort St

Site Code: 2204400001

Municipality: Amherstburg

Count Date: Feb 24, 2022

East Approach - Fort St

			Cars				T	rucks			Buses					Bi	cycles			Total	
Start Time	4	1	•	J	Total	4	1	•	J	Total	4	1	•	1	Total	4	1	•	1	Total	
16:00	13	0	3	0	16	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
16:15	15	0	10	0	25	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
16:30	1	0	5	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
16:45	11	0	4	0	15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	C
17:00	9	0	2	0	11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
17:15	4	0	3	0	7	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	C
17:30	11	0	3	0	14	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
17:45	7	0	1	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	C
SUBTOTAL	71	0	31	0	102	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8
GRAND TOTAL	89	0	53	0	142	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	14



Peak Hour Diagram

Specified Period

One Hour Peak

From: To: 07:00:00 09:00:00

From: 08:00:00 To: 09:00:00

Intersection:

Sandwich St S & Fort St

 Site Code:
 2204400001

 Count Date:
 Feb 24, 2022

Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Sandwich St S runs N/S

North Approach

	Out	In	Total
	455	339	794
	12	8	20
	7	3	10
ॐ	0	0	0
	474	350	824

Sandwich St S

		E.	LÎ.
Totals	454	20	0
	435	20	0
	12	0	0
	7	0	0
<i>₫</i>	0	0	0

East Approach

	Out	In	Total
	26	65	91
	1	1	2
	0	0	0
ॐ	0	0	0
	27	66	93

Peds: 1



Peds: 5



otals				<i>₫</i>
0	0	0	0	0
15	15	0	0	0
12	11	1	0	0

Peds: 0

	1	•	J.
Totals	335	46	0
	324	45	0
	8	1	0
	3	0	0
₫ %	0	0	0

Sandwich St S

South Approach

	Out	ln	Total
	369	446	815
	9	13	22
	3	7	10
<i>₫</i> 6	0	0	0
	381	466	847

📾 - Cars

🚨 - Trucks

🖽 - Buses

♣ - Bicycles

Comments



Peak Hour Summary

Sandwich St S & Fort St Intersection:

Site Code: 2204400001 Count Date: Feb 24, 2022 Period: 07:00 - 09:00

Peak Hour Data (08:00 - 09:00)

		ľ	North A Sandw	Approac	:h			S	outh A Sandw	pproac	h				East Ap For	pproach rt St	1				West A	pproach	1		Total Vehicl
Start Time	4	1	•	J	Peds	Total	4	1	•	O.	Peds	Total	•	1	•	•	Peds	Total	4	1	•	J	Peds	Total	es
08:00	11	99		0	0	110		107	9	0	0	116	1		5	0	2	6					0		232
08:15	7	120		0	0	127		73	21	0	0	94	6		5	0	1	11					0		232
08:30	1	103		0	0	104		80	6	0	0	86	5		2	0	0	7					0		197
08:45	1	132		0	1	133		75	10	0	0	85	0		3	0	2	3					0		221
Grand Total	20	454		0	1	474		335	46	0	0	381	12		15	0	5	27					0	0	882
Approach %	4.2	95.8		0		-		87.9	12.1	0		-	44.4		55.6	0		-						-	
Totals %	2.3	51.5		0		53.7		38	5.2	0		43.2	1.4		1.7	0		3.1						0	
PHF	0.45	0.86		0		0.89		0.78	0.55	0		0.82	0.5		0.75	0		0.61						0	0.95
Cars	20	435		0		455		324	45	0		369	11		15	0		26						0	850
% Cars	100	95.8		0		96		96.7	97.8	0		96.9	91.7		100	0		96.3						0	96.4
Trucks	0	12		0		12		8	1	0		9	1		0	0		1						0	22
% Trucks	0	2.6		0		2.5		2.4	2.2	0		2.4	8.3		0	0		3.7						0	2.5
Buses	0	7		0		7		3	0	0		3	0		0	0		0						0	10
% Buses	0	1.5		0		1.5		0.9	0	0		0.8	0		0	0		0						0	1.1
Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
% Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
Peds					1	-					0	-					5	-					0	-	6
% Peds					16.7	-					0	-					83.3	-					0	-	



Peak Hour Diagram

Specified Period

One Hour Peak

From: 16:00:00 To: 18:00:00 From: 16:15:00 To: 17:15:00

Intersection: Sandwich St S & Fort St

 Site Code:
 2204400001

 Count Date:
 Feb 24, 2022

Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Sandwich St S runs N/S

North Approach

	Out	In	Total
	572	563	1135
	3	8	11
	0	0	0
<i>₫</i>	0	0	0
	575	571	1146

Sandwich St S

		E.	.1
Totals	563	12	0
	560	12	0
	3	0	0
	0	0	0
ॐ	0	0	0

East Approach

	Out	In	Total
	57	80	137
	0	0	0
	0	0	0
ॐ	0	0	0
	57	80	137

Fort St

Peds: 0



Peds: 5

<u>To</u>

 Totals
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Peds: 1

	1		J
Totals	550	68	0
	542	68	0
	8	0	0
	0	0	0
₫ %	0	0	0

Sandwich St S

South Approach

	Out	ln	Total
	610	596	1206
	8	3	11
田	0	0	0
<i>₫</i> 6	0	0	0
	618	599	1217

📾 - Cars

🚨 - Trucks

🖽 - Buses

♣ - Bicycles

Comments



Peak Hour Summary

Intersection: Sandwich St S & Fort St

 Site Code:
 2204400001

 Count Date:
 Feb 24, 2022

 Period:
 16:00 - 18:00

Peak Hour Data (16:15 - 17:15)

		ı	North A Sandw	pproac	h			S	outh A Sandw	pproac	h				East Ap For	proach t St	1				West A	pproac	h		Total Vehicl
Start Time	4	1	•	4	Peds	Total	4	1	•	4	Peds	Total	•	1	•	4	Peds	Total	4	1	•	1	Peds	Total	es
16:15	5	148		0	0	153		124	17	0	0	141	15		10	0	3	25					0		319
16:30	1	147		0	0	148		140	17	0	1	157	1		5	0	1	6					0		311
16:45	5	141		0	0	146		139	22	0	0	161	11		4	0	0	15					0		322
17:00	1	127		0	0	128		147	12	0	0	159	9		2	0	1	11					0		298
Grand Total	12	563		0	0	575		550	68	0	1	618	36		21	0	5	57					0	0	1250
Approach %	2.1	97.9		0		-		89	11	0		-	63.2		36.8	0		-						-	
Totals %	1	45		0		46		44	5.4	0		49.4	2.9		1.7	0		4.6						0	
PHF	0.6	0.95		0		0.94		0.94	0.77	0		0.96	0.6		0.53	0		0.57						0	0.97
Cars	12	560		0		572		542	68	0		610	36		21	0		57						0	1239
% Cars	100	99.5		0		99.5		98.5	100	0		98.7	100		100	0		100						0	99.1
Trucks	0	3		0		3		8	0	0		8	0		0	0		0						0	11
% Trucks	0	0.5		0		0.5		1.5	0	0		1.3	0		0	0		0						0	0.9
Buses	0	0		0		0		0	0	0		0	0		0	0		0						0	0
% Buses	0	0		0		0		0	0	0		0	0		0	0		0						0	0
Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
% Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
Peds					0	-					1	-					5	-					0	-	6
% Peds					0	-					16.7	-					83.3	-					0	-	



Project #22-044 - CGH Transportation

Intersection Count Report

Intersection: Sandwich St S & Parking Lot Driveway

Municipality: Amherstburg

Count Date: Feb 24, 2022

Site Code: 2204400002

Count Categories: Cars, Trucks, Buses, Bicycles, Pedestrians

Count Period: 07:00-09:00, 16:00-18:00

Weather: Clear

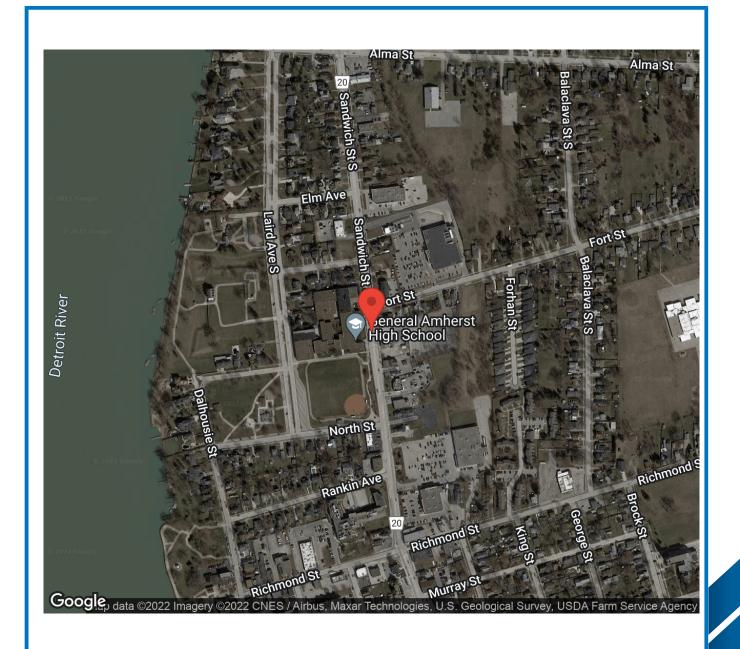


Traffic Count Map

Intersection: Sandwich St S & Parking Lot Driveway

Site Code: 2204400002 Municipality: Amherstburg

Count Date: Feb 24, 2022





Traffic Count Summary

Intersection: Sandwich St S & Parking Lot Driveway

Site Code: 2204400002

Municipality: Amherstburg

Count Date: Feb 24, 2022

Sandwich St S - Traffic Summary

		North	Appr	oach T	otals			South	Appr	oach T	otals				
	Inc	cludes Ca	ars, Truc	ks, Buse	s, Bicycle	s	Inc	cludes Ca	rs, Truc	ks, Buses	s, Bicycle	S			
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total		
07:00 - 08:00	8	243	0	0	251	0	0	325	5	0	330	36	581		
08:00 - 09:00	13	453	0	0	466	0	0	362	27	0	389	137	855		
	BREAK														
16:00 - 17:00	2	618	0	0	620	0	0	589	5	0	594	9	1214		
17:00 - 18:00	3	525	0	0	528	1	0	510	4	0	514	4	1042		
GRAND TOTAL	26	1839	0	0	1865	1	0	1786	41	0	1827	186	3692		



Traffic Count Summary

Intersection: Sandwich St S & Parking Lot Driveway

Site Code: 2204400002

Municipality: Amherstburg

Count Date: Feb 24, 2022

Parking Lot Driveway - Traffic Summary

		East /	Appro	ach To			West	Appro	oach To	otals					
	Inc	ludes Ca	rs, Truc	ks, Buses	Inc	ludes Ca	rs, Truc	ks, Buses	, Bicycle	S					
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total		
07:00 - 08:00	1	0	3	0	4	0	0	0	0	0	0	0	4		
08:00 - 09:00	1	0	19	0	20	19	0	0	0	0	0	0	20		
	BREAK														
16:00 - 17:00	3	0	3	0	6	6	0	0	0	0	0	0	(
17:00 - 18:00	1	0	3	0	4	5	0	0	0	0	0	0	4		
GRAND TOTAL	6	0	28	0	34	30	0	0	0	0	0	0	34		



Intersection: Sandwich St S & Parking Lot Driveway

Site Code: 2204400002

Municipality: Amherstburg

Count Date: Feb 24, 2022

North Approach - Sandwich St S

			Cars				Т	rucks				E	Buses				В	icycles			Total
Start Time	4	1	•	1	Total	4	1	•	J	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	0	37	0	0	37	0	2	0	0	2	0	1	0	0	1	0	0	0	0	0	0
07:15	0	48	0	0	48	0	2	0	0	2	0	2	0	0	2	0	0	0	0	0	0
07:30	3	60	0	0	63	0	4	0	0	4	0	5	0	0	5	0	0	0	0	0	0
07:45	5	77	0	0	82	0	2	0	0	2	0	3	0	0	3	0	0	0	0	0	0
08:00	5	91	0	0	96	0	1	0	0	1	0	3	0	0	3	0	0	0	0	0	0
08:15	5	117	0	0	122	0	2	0	0	2	0	2	0	0	2	0	0	0	0	0	0
08:30	2	102	0	0	104	0	4	0	0	4	0	0	0	0	0	0	0	0	0	0	0
08:45	1	123	0	0	124	0	6	0	0	6	0	2	0	0	2	0	0	0	0	0	0
SUBTOTAL	21	655	0	0	676	0	23	0	0	23	0	18	0	0	18	0	0	0	0	0	0



Intersection: Sandwich St S & Parking Lot Driveway

Site Code: 2204400002

Municipality: Amherstburg

Count Date: Feb 24, 2022

North Approach - Sandwich St S

			Cars				Tı	rucks				ı	Buses				В	icycles			T.4.1
Start Time	4	1	•	1	Total	4	1	•	Q	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
16:00	2	153	0	0	155	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0
16:15	0	161	0	0	161	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0
16:30	0	148	0	0	148	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	0	151	0	0	151	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
17:00	0	136	0	0	136	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
17:15	0	137	0	0	137	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
17:30	1	125	0	0	126	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
17:45	2	124	0	0	126	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0
SUBTOTAL	5	1135	0	0	1140	0	5	0	0	5	0	3	0	0	3	0	0	0	0	0	1
GRAND TOTAL	26	1790	0	0	1816	0	28	0	0	28	0	21	0	0	21	0	0	0	0	0	1



Intersection: Sandwich St S & Parking Lot Driveway

Site Code: 2204400002

Municipality: Amherstburg

Count Date: Feb 24, 2022

South Approach - Sandwich St S

			Cars				T	rucks				В	Buses				Bi	icycles			Total
Start Time	4	1	•	1	Total	4	1	•	J.	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	0	67	0	0	67	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	1
07:15	0	76	1	0	77	0	4	0	0	4	0	3	0	0	3	0	0	0	0	0	1
07:30	0	87	1	0	88	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	9
07:45	0	78	3	0	81	0	4	0	0	4	0	1	0	0	1	0	0	0	0	0	25
08:00	0	99	13	0	112	0	3	0	0	3	0	3	0	0	3	0	0	0	0	0	61
08:15	0	88	9	0	97	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	64
08:30	0	81	2	0	83	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	5
08:45	0	82	3	0	85	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	7
SUBTOTAL	0	658	32	0	690	0	20	0	0	20	0	9	0	0	9	0	0	0	0	0	173



Intersection: Sandwich St S & Parking Lot Driveway

Site Code: 2204400002

Municipality: Amherstburg

Count Date: Feb 24, 2022

South Approach - Sandwich St S

			Cars				Tı	rucks				В	Buses				Bi	cycles			Total
Start Time	4	1	•	1	Total	4	1	•	J.	Total	4	1	•	1	Total	4	1	•	1	Total	
16:00	0	131	2	0	133	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	5
16:15	0	138	1	0	139	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	2
16:30	0	155	2	0	157	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	2
16:45	0	158	0	0	158	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0
17:00	0	157	2	0	159	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0
17:15	0	133	1	0	134	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	0	111	0	0	111	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45	0	106	1	0	107	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	4
SUBTOTAL	0	1089	9	0	1098	0	9	0	0	9	0	1	0	0	1	0	0	0	0	0	13
GRAND TOTAL	0	1747	41	0	1788	0	29	0	0	29	0	10	0	0	10	0	0	0	0	0	186



Intersection: Sandwich St S & Parking Lot Driveway

Site Code: 2204400002

Municipality: Amherstburg

Count Date: Feb 24, 2022

East Approach - Parking Lot Driveway

	Cars					Trucks			Buses				Bicycles					Total			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45	1	0	1	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	11	0	11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9
08:15	0	0	5	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8
08:30	0	0	3	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
08:45	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	2	0	22	0	24	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	19



Intersection: Sandwich St S & Parking Lot Driveway

Site Code: 2204400002

Municipality: Amherstburg

Count Date: Feb 24, 2022

East Approach - Parking Lot Driveway

			Cars			Trucks			Buses				Bicycles					Total			
Start Time	4	1		1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
16:00	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
16:15	1	0	2	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
16:30	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
17:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	1	0	1	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
17:45	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4
SUBTOTAL	4	0	6	0	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11
GRAND TOTAL	6	0	28	0	34	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	30



Peak Hour Diagram

Specified Period

One Hour Peak

From: To: 07:00:00 09:00:00

From: 08:00:00 To: 09:00:00

Intersection: Sandwich St S & Parking Lot Driveway

 Site Code:
 2204400002

 Count Date:
 Feb 24, 2022

Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Sandwich St S runs N/S

North Approach

	Out	In	Total
	446	369	815
	13	9	22
	7	3	10
<i>₫</i>	0	0	0
	466	381	847

Sandwich St S

		E.	. 1
Totals	453	13	0
	433	13	0
	13	0	0
	7	0	0
<i>₫</i>	0	0	0

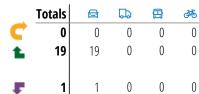
East Approach

	Out	In	Total
	20	40	60
	0	0	0
	0	0	0
₫ %	0	0	0
	20	40	60

Peds: 0



Parking Lot Driveway



Peds: 137

1	•	J
362	27	0
350	27	0
9	0	0
3	0	0
0	0	0
	350 9 3	350 27 9 0 3 0

Sandwich St S

South Approach

	Out	In	Total
	377	434	811
	9	13	22
圕	3	7	10
<i>₫</i> 6	0	0	0
	389	454	843





🖽 - Buses

Bicycles

Comments



Peak Hour Summary

Intersection: Sandwich St S & Parking Lot Driveway

 Site Code:
 2204400002

 Count Date:
 Feb 24, 2022

 Period:
 07:00 - 09:00

Peak Hour Data (08:00 - 09:00)

				Approac vich St S						pproac				Parl	East Ap king Lo	proach t Drive	n way		West Approach						
Start Time	4	1	•	J	Peds	Total	4	1	•	4	Peds	Total	4	1	•	•	Peds	Total	4	1		1	Peds	Total	Vehicl es
08:00	5	95		0	0	100		105	13	0	61	118	0		11	0	9	11					0		229
08:15	5	121		0	0	126		89	9	0	64	98	0		5	0	8	5					0		229
08:30	2	106		0	0	108		83	2	0	5	85	0		3	0	2	3					0		196
08:45	1	131		0	0	132		85	3	0	7	88	1		0	0	0	1					0		221
Grand Total	13	453		0	0	466		362	27	0	137	389	1		19	0	19	20					0	0	875
Approach %	2.8	97.2		0		-		93.1	6.9	0		-	5		95	0		-						-	
Totals %	1.5	51.8		0		53.3		41.4	3.1	0		44.5	0.1		2.2	0		2.3						0	
PHF	0.65	0.86		0		0.88		0.86	0.52	0		0.82	0.25		0.43	0		0.45						0	0.96
Cars	13	433		0		446		350	27	0		377	1		19	0		20						0	843
% Cars	100	95.6		0		95.7		96.7	100	0		96.9	100		100	0		100						0	96.3
Trucks	0	13		0		13		9	0	0		9	0		0	0		0						0	22
% Trucks	0	2.9		0		2.8		2.5	0	0		2.3	0		0	0		0						0	2.5
Buses	0	7		0		7		3	0	0		3	0		0	0		0						0	10
% Buses	0	1.5		0		1.5		0.8	0	0		0.8	0		0	0		0						0	1.1
Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
% Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
Peds					0	-					137	-					19	-					0	-	156
% Peds					0	-					87.8	-					12.2	-					0	-	



Peak Hour Diagram

Specified Period

One Hour Peak

To:

From: 16:00:00 To: 18:00:00 From: 16:15:00

17:15:00

Intersection: Sandwich St S & Parking Lot Driveway

 Site Code:
 2204400002

 Count Date:
 Feb 24, 2022

Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Sandwich St S runs N/S

North Approach

	Out	In	Total
	596	610	1206
	3	8	11
	0	0	0
<i>₫</i>	0	0	0
	599	618	1217

Sandwich St S

		E.	LÎ.
Totals	599	0	0
	596	0	0
	3	0	0
	0	0	0
<i>₫</i>	0	0	0

East Approach

	Out	In	Total
	5	5	10
	0	0	0
	0	0	0
₫ %	0	0	0
	5	5	10

Peds: 1





Parking Lot Driveway

	Totals				₫ %
C	0	0	0	0	0
£	2	2	0	0	0
F	3	3	0	0	0

Peds: 4

	1		J
Totals	616	5	0
	608	5	0
	8	0	0
	0	0	0
<i>₫</i>	0	0	0

Sandwich St S

South Approach

	Out	In	Total
	613	599	1212
	8	3	11
	0	0	0
ॐ	0	0	0
	621	602	1223







♣ - Bicycles

Comments



Peak Hour Summary

Intersection: Sandwich St S & Parking Lot Driveway

 Site Code:
 2204400002

 Count Date:
 Feb 24, 2022

 Period:
 16:00 - 18:00

Peak Hour Data (16:15 - 17:15)

		ı	North A Sandw	opproac	h			S	outh A Sandw	pproac	h			Par	East Ap king Lo	proach t Drive	n way				West A	pproac	h		Total Vehicl
Start Time	4	1	•	4	Peds	Total	4	1	•	4	Peds	Total	4	1	•	4	Peds	Total	4	1	•	1	Peds	Total	es
16:15	0	163		0	0	163		139	1	0	2	140	1		2	0	3	3					0		306
16:30	0	148		0	0	148		157	2	0	2	159	1		0	0	0	1					0		308
16:45	0	152		0	0	152		161	0	0	0	161	1		0	0	1	1					0		314
17:00	0	136		0	1	136		159	2	0	0	161	0		0	0	0	0					0		297
Grand Total	0	599		0	1	599		616	5	0	4	621	3		2	0	4	5					0	0	1225
Approach %	0	100		0		-		99.2	0.8	0		-	60		40	0		-						-	
Totals %	0	48.9		0		48.9		50.3	0.4	0		50.7	0.2		0.2	0		0.4						0	
PHF	0	0.92		0		0.92		0.96	0.63	0		0.96	0.75		0.25	0		0.42						0	0.98
Cars	0	596		0		596		608	5	0		613	3		2	0		5						0	1214
% Cars	0	99.5		0		99.5		98.7	100	0		98.7	100		100	0		100						0	99.1
Trucks	0	3		0		3		8	0	0		8	0		0	0		0						0	11
% Trucks	0	0.5		0		0.5		1.3	0	0		1.3	0		0	0		0						0	0.9
Buses	0	0		0		0		0	0	0		0	0		0	0		0						0	0
% Buses	0	0		0		0		0	0	0		0	0		0	0		0						0	0
Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
% Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
Peds					1	-					4	-					4	-					0	-	9
% Peds					11.1	-					44.4	-					44.4	-					0	-	



Project #22-044 - CGH Transportation

Intersection Count Report

Intersection: Sandwich St S & North St-165 Sandwich St S Driveway

Municipality: Amherstburg

Count Date: Feb 24, 2022

Site Code: 2204400003

Count Categories: Cars, Trucks, Buses, Bicycles, Pedestrians

Count Period: 07:00-09:00, 16:00-18:00

Weather: Clear



Traffic Count Map

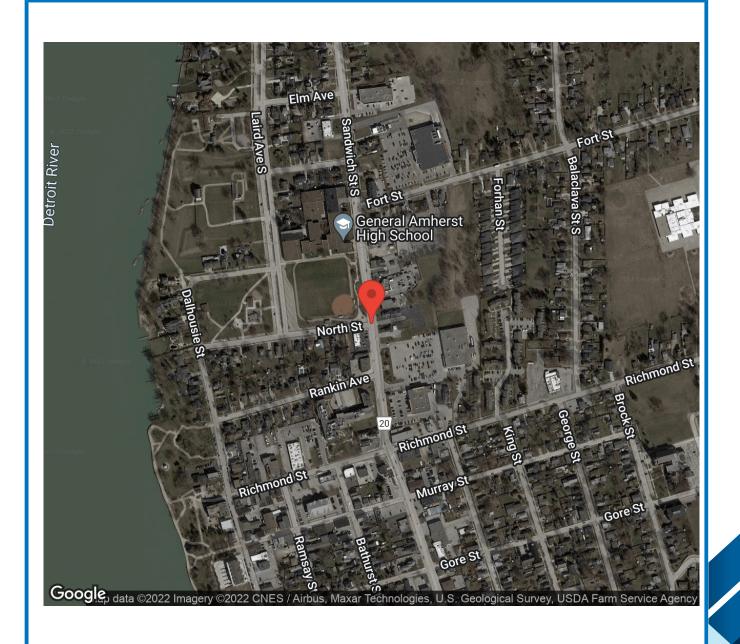
Intersection: Sandwich St S & North St-165 Sandwich St

S Driveway

Site Code: 2204400003

Municipality: Amherstburg

Count Date: Feb 24, 2022



Traffic Count Summary



Intersection: Sandwich St & North St-165 Sandwich St

S Driveway

Site Code: 2204400003 Municipality: Amherstburg Count Date: Feb 24, 2022

Sandwich St S - Traffic Summary

		North	Appr	oach T	otals			South	Appr	oach T	otals		
	Inc	cludes Ca	rs, Truc	ks, Buse	s, Bicycle	s	Inc	cludes Ca	rs, Truc	ks, Buses	s, Bicycle	S	
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	5	222	11	0	238	0	5	326	3	0	334	0	572
08:00 - 09:00	5	432	23	0	460	0	53	382	4	0	439	1	899
					В	REAK							
16:00 - 17:00	2	609	9	0	620	0	2	589	0	0	591	0	1211
17:00 - 18:00	0	525	4	0	529	0	3	515	0	0	518	1	1047
GRAND TOTAL	12	1788	47	0	1847	0	63	1812	7	0	1882	2	3729

Traffic Count Summary



Intersection: Sandwich St & North St-165 Sandwich St

S Driveway

Site Code: 2204400003 Municipality: Amherstburg

Count Date: Feb 24, 2022

165 Sandwich St S Driveway - Traffic Summary

		East	Appro	ach To	tals			West	Appro	oach To	otals		
	Inc	ludes Ca	ars, Truc	ks, Buses	s, Bicycle	S	Inc	ludes Ca	rs, Truc	ks, Buses	s, Bicycle	S	
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	0	0	0	0	0	4	6	0	1	0	7	1	7
08:00 - 09:00	1	0	0	0	1	26	2	0	15	0	17	9	18
					В	REAK							
16:00 - 17:00	3	0	3	0	6	1	2	0	3	0	5	6	11
17:00 - 18:00	2	0	1	0	3	1	1	0	4	0	5	4	8
GRAND TOTAL	6	0	4	0	10	32	11	0	23	0	34	20	44



Intersection: Sandwich St S & North St-165 Sandwich St S Driveway

Site Code: 2204400003

Municipality: Amherstburg

Count Date: Feb 24, 2022

North Approach - Sandwich St S

			Cars				Tı	rucks				В	uses				Bi	cycles			Total
Start Time	4	1	•	1	Total	4	1	•	J.	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	0	37	2	0	39	0	2	0	0	2	0	1	0	0	1	0	0	0	0	0	0
07:15	1	43	2	0	46	0	2	0	0	2	0	2	0	0	2	0	0	0	0	0	0
07:30	2	55	1	0	58	0	4	0	0	4	0	3	2	0	5	0	0	0	0	0	0
07:45	2	71	2	0	75	0	1	0	0	1	0	1	2	0	3	0	0	0	0	0	0
08:00	0	84	12	0	96	0	2	0	0	2	0	2	1	0	3	0	0	0	0	0	0
08:15	0	111	5	0	116	0	2	0	0	2	0	2	0	0	2	0	0	0	0	0	0
08:30	2	99	2	0	103	0	5	0	0	5	0	0	0	0	0	0	0	0	0	0	0
08:45	3	118	3	0	124	0	5	0	0	5	0	2	0	0	2	0	0	0	0	0	0
SUBTOTAL	10	618	29	0	657	0	23	0	0	23	0	13	5	0	18	0	0	0	0	0	0



Intersection: Sandwich St S & North St-165 Sandwich St S Driveway

Site Code: 2204400003

Municipality: Amherstburg

Count Date: Feb 24, 2022

North Approach - Sandwich St S

			Cars				Tı	rucks				В	uses				Bi	icycles			Total
Start Time	4	1	•	1	Total	4	1	•	J.	Total	4	1	•	1	Total	4	1	P	1	Total	Total Peds
16:00	0	148	3	0	151	0	0	0	0	0	0	1	1	0	2	0	0	0	0	0	0
16:15	1	159	1	0	161	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
16:30	1	147	2	0	150	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
16:45	0	151	2	0	153	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
17:00	0	134	1	0	135	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	0	138	1	0	139	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0
17:30	0	126	1	0	127	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
17:45	0	123	1	0	124	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0
SUBTOTAL	2	1126	12	0	1140	0	6	0	0	6	0	2	1	0	3	0	0	0	0	0	0
GRAND TOTAL	12	1744	41	0	1797	0	29	0	0	29	0	15	6	0	21	0	0	0	0	0	0



Intersection: Sandwich St S & North St-165 Sandwich St S Driveway

Site Code: 2204400003

Municipality: Amherstburg

Count Date: Feb 24, 2022

South Approach - Sandwich St S

			Cars				Ti	rucks				В	uses				Bi	cycles			Total
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	-	1	Total	Total Peds
07:00	2	68	0	0	70	0	1	0	0	1	0	2	0	0	2	0	0	0	0	0	0
07:15	0	74	0	0	74	0	3	0	0	3	0	3	0	0	3	0	0	0	0	0	0
07:30	0	82	0	0	82	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0
07:45	3	85	3	0	91	0	4	0	0	4	0	1	0	0	1	0	0	0	0	0	0
08:00	22	112	2	0	136	2	2	0	0	4	2	3	0	0	5	0	0	0	0	0	0
08:15	21	94	1	0	116	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	1
08:30	5	83	1	0	89	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0
08:45	1	82	0	0	83	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	54	680	7	0	741	2	19	0	0	21	2	9	0	0	11	0	0	0	0	0	1



Intersection: Sandwich St S & North St-165 Sandwich St S Driveway

Site Code: 2204400003

Municipality: Amherstburg

Count Date: Feb 24, 2022

South Approach - Sandwich St S

			Cars				Tı	rucks				В	uses				Bi	cycles			Tatal
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
16:00	1	135	0	0	136	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0
16:15	1	138	0	0	139	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0
16:30	0	157	0	0	157	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
16:45	0	152	0	0	152	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0
17:00	2	160	0	0	162	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0
17:15	0	132	0	0	132	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	1	110	0	0	111	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1
17:45	0	109	0	0	109	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	5	1093	0	0	1098	0	10	0	0	10	0	1	0	0	1	0	0	0	0	0	1
GRAND TOTAL	59	1773	7	0	1839	2	29	0	0	31	2	10	0	0	12	0	0	0	0	0	2



Intersection: Sandwich St S & North St-165 Sandwich St S Driveway

Site Code: 2204400003

Municipality: Amherstburg

Count Date: Feb 24, 2022

East Approach - 165 Sandwich St S Driveway

			Cars				Ti	rucks				В	uses				Bi	cycles			T. 4.1
Start Time	4	1	•	1	Total	4	1	•	Q	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
07:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	15
08:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8
08:30	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
08:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
SUBTOTAL	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	30



Intersection: Sandwich St S & North St-165 Sandwich St S Driveway

Site Code: 2204400003

Municipality: Amherstburg

Count Date: Feb 24, 2022

East Approach - 165 Sandwich St S Driveway

			Cars				Tı	rucks				В	uses				Bi	cycles			Takal
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
16:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	1	0	1	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
16:45	1	0	1	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
17:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	5	0	4	0	9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
GRAND TOTAL	6	0	4	0	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	32



Intersection: Sandwich St S & North St-165 Sandwich St S Driveway

Site Code: 2204400003

Municipality: Amherstburg

Count Date: Feb 24, 2022

West Approach - North St

			Cars				Ti	rucks				В	uses				Bi	cycles			Total
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	(
07:15	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	(
07:30	3	0	1	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	(
07:45	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
08:00	2	0	8	0	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4
08:15	0	0	4	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
08:30	0	0	3	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	(
08:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
SUBTOTAL	8	0	16	0	24	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1(



Intersection: Sandwich St S & North St-165 Sandwich St S Driveway

Site Code: 2204400003

Municipality: Amherstburg

Count Date: Feb 24, 2022

West Approach - North St

		(Cars				Tı	rucks				В	uses				Bi	cycles			Total
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
16:00	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
16:30	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
16:45	1	0	1	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
17:00	1	0	2	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
17:15	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
17:30	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
17:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	3	0	7	0	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10
GRAND TOTAL	11	0	23	0	34	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	20



Peak Hour Diagram

Specified Period

One Hour Peak

From: To: 07:00:00 09:00:00

From: 08:00:00 To: 09:00:00

-

Sandwich St S & North St-165 Sandwich St S Driveway

 Site Code:
 2204400003

 Count Date:
 Feb 24, 2022

Intersection:

Weather conditions:

Clear

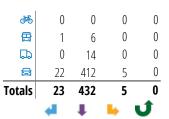
** Unsignalized Intersection **

Major Road: Sandwich St S runs N/S

North Approach

	Out	In	Total
	439	373	812
	14	8	22
	7	3	10
<i>₫</i>	0	0	0
	460	384	844

Sandwich St S



East Approach

	Out	ln	Total
	1	9	10
	0	0	0
	0	0	0
₹	0	0	0
	1	9	10

North St

	Totals				<i>₫</i>
7	0	0	0	0	0
4	2	2	0	0	0
-	0	0	0	0	0
4	15	15	0	0	0

Peds: 0



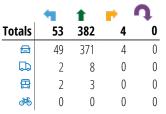
165 Sandwich St S Driveway

	Totals				<i>₫</i>
C	0	0	0	0	0
£	0	0	0	0	0
-	0	0	0	0	0
F	1	1	0	0	0

West Approach

	Out	ln	Total
	17	71	88
	0	2	2
	0	3	3
<i>₫</i>	0	0	0
	17	76	93

Peds



Peds: 1

Sandwich St S

South Approach

	Out	In	Total
	424	428	852
	10	14	24
	5	6	11
æ€	0	0	0
	439	448	887







♣ - Bicycles

Comments



Peak Hour Summary

Intersection: Sandwich St S & North St-165 Sandwich St S Driveway

 Site Code:
 2204400003

 Count Date:
 Feb 24, 2022

 Period:
 07:00 - 09:00

Peak Hour Data (08:00 - 09:00)

		N	lorth A Sandw	pproac	:h			S	outh A Sandw	pproac	h			165 Sa	East A _l ndwich	pproacl 1 St S D	h riveway	1			West A Nor	pproach th St	1		Total Vehicl
Start Time	4	1	P	J	Peds	Total	4	1	P	J	Peds	Total	4	1	P	J	Peds	Total	4	1	•	1	Peds	Total	es
08:00	0	88	13	0	0	101	26	117	2	0	0	145	0	0	0	0	15	0	2	0	8	0	4	10	256
08:15	0	115	5	0	0	120	21	96	1	0	1	118	0	0	0	0	8	0	0	0	4	0	3	4	242
08:30	2	104	2	0	0	108	5	85	1	0	0	91	1	0	0	0	1	1	0	0	3	0	0	3	203
08:45	3	125	3	0	0	131	1	84	0	0	0	85	0	0	0	0	2	0	0	0	0	0	2	0	216
Grand Total	5	432	23	0	0	460	53	382	4	0	1	439	1	0	0	0	26	1	2	0	15	0	9	17	917
Approach %	1.1	93.9	5	0		-	12.1	87	0.9	0		-	100	0	0	0		-	11.8	0	88.2	0		-	
Totals %	0.5	47.1	2.5	0		50.2	5.8	41.7	0.4	0		47.9	0.1	0	0	0		0.1	0.2	0	1.6	0		1.9	
PHF	0.42	0.86	0.44	0		0.88	0.51	0.82	0.5	0		0.76	0.25	0	0	0		0.25	0.25	0	0.47	0		0.43	0.9
Cars	5	412	22	0		439	49	371	4	0		424	1	0	0	0		1	2	0	15	0		17	881
% Cars	100	95.4	95.7	0		95.4	92.5	97.1	100	0		96.6	100	0	0	0		100	100	0	100	0		100	96.1
Trucks	0	14	0	0		14	2	8	0	0		10	0	0	0	0		0	0	0	0	0		0	24
% Trucks	0	3.2	0	0		3	3.8	2.1	0	0		2.3	0	0	0	0		0	0	0	0	0		0	2.6
Buses	0	6	1	0		7	2	3	0	0		5	0	0	0	0		0	0	0	0	0		0	12
% Buses	0	1.4	4.3	0		1.5	3.8	0.8	0	0		1.1	0	0	0	0		0	0	0	0	0		0	1.3
Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
Peds					0	-					1	-					26	-					9	-	36
% Peds					0	-					2.8	-					72.2	-					25	-	



Peak Hour Diagram

Specified Period

One Hour Peak

From: To: 16:00:00 18:00:00 From: 16:15:00 To: 17:15:00

Intersection: Sandwich St S & North St-165 Sandwich St S Driveway

 Site Code:
 2204400003

 Count Date:
 Feb 24, 2022

Weather conditions:

Clear

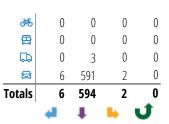
** Unsignalized Intersection **

Major Road: Sandwich St S runs N/S

North Approach

	Out	In	Total
	599	613	1212
	3	8	11
	0	0	0
<i>₫</i>	0	0	0
	602	621	1223

Sandwich St S



East Approach

	Out	ln	Total
	6	2	8
	0	0	0
	0	0	0
₩	0	0	0
	6	2	8

North St

	Totals				₫ %
7	0	0	0	0	0
4	3	3	0	0	0
\rightarrow	0	0	0	0	0
4	4	4	0	0	0





165 Sandwich St S Driveway

	Totals				<i>₫</i>
C	0	0	0	0	0
£	3	3	0	0	0
-	0	0	0	0	0
F	3	3	0	0	0

West Approach

	Out	In	Total
	7	9	16
	0	0	0
	0	0	0
<i>₫</i>	0	0	0
	7	9	16

	4	t	•	J
Totals	3	615	0	0
	3	607	0	0
₽	0	8	0	0
毌	0	0	0	0
<i>₫</i> %	0	0	0	0

Peds: 0

Sandwich St S

South Approach

	Out	ln	Total
	610	598	1208
	8	3	11
	0	0	0
<i>₫</i>	0	0	0
	618	601	1219







♣ - Bicycles

Comments



Peak Hour Summary

Intersection: Sandwich St S & North St-165 Sandwich St S Driveway

 Site Code:
 2204400003

 Count Date:
 Feb 24, 2022

 Period:
 16:00 - 18:00

Peak Hour Data (16:15 - 17:15)

		N	North <i>A</i> Sandw	opproach pich St S	ch S			S	outh <i>A</i> Sandw	Approac vich St S	h			165 Sa	East Ap ndwich	pproacl St S D	า rivewa	У			West A Nor	pproacl th St	h		Total Vehicl
Start Time	4	1	P	4	Peds	Total	4	1	•	1	Peds	Total	4	1	•	1	Peds	Total	4	1	•	4	Peds	Total	es
16:15	1	160	1	0	0	162	1	140	0	0	0	141	1	0	1	0	0	2	0	0	1	0	2	1	306
16:30	1	148	2	0	0	151	0	158	0	0	0	158	0	0	1	0	1	1	1	0	0	0	1	1	311
16:45	0	152	2	0	0	154	0	155	0	0	0	155	1	0	1	0	0	2	1	0	1	0	3	2	313
17:00	0	134	1	0	0	135	2	162	0	0	0	164	1	0	0	0	0	1	1	0	2	0	2	3	303
Grand Total	2	594	6	0	0	602	3	615	0	0	0	618	3	0	3	0	1	6	3	0	4	0	8	7	1233
Approach %	0.3	98.7	1	0		-	0.5	99.5	0	0		-	50	0	50	0		-	42.9	0	57.1	0		-	
Totals %	0.2	48.2	0.5	0		48.8	0.2	49.9	0	0		50.1	0.2	0	0.2	0		0.5	0.2	0	0.3	0		0.6	
PHF	0.5	0.93	0.75	0		0.93	0.38	0.95	0	0		0.94	0.75	0	0.75	0		0.75	0.75	0	0.5	0		0.58	0.98
Cars	2	591	6	0		599	3	607	0	0		610	3	0	3	0		6	3	0	4	0		7	1222
% Cars	100	99.5	100	0		99.5	100	98.7	0	0		98.7	100	0	100	0		100	100	0	100	0		100	99.1
Trucks	0	3	0	0		3	0	8	0	0		8	0	0	0	0		0	0	0	0	0		0	11
% Trucks	0	0.5	0	0		0.5	0	1.3	0	0		1.3	0	0	0	0		0	0	0	0	0		0	0.9
Buses	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Buses	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
Peds					0	-					0	-					1	-					8	-	9
% Peds					0	-					0	-					11.1	-					88.9	-	



Project #21-078 - Golder Associates

Intersection Count Report

Intersection: Balaclava St S & Fort St

Municipality: Amherstburg

Count Date: Jun 16, 2021

Site Code: 2107800021

Count Categories: Cars, Trucks, Bicycles, Pedestrians

Count Period: 07:00-09:00, 11:00-14:00, 15:00-18:00

Weather: Clear

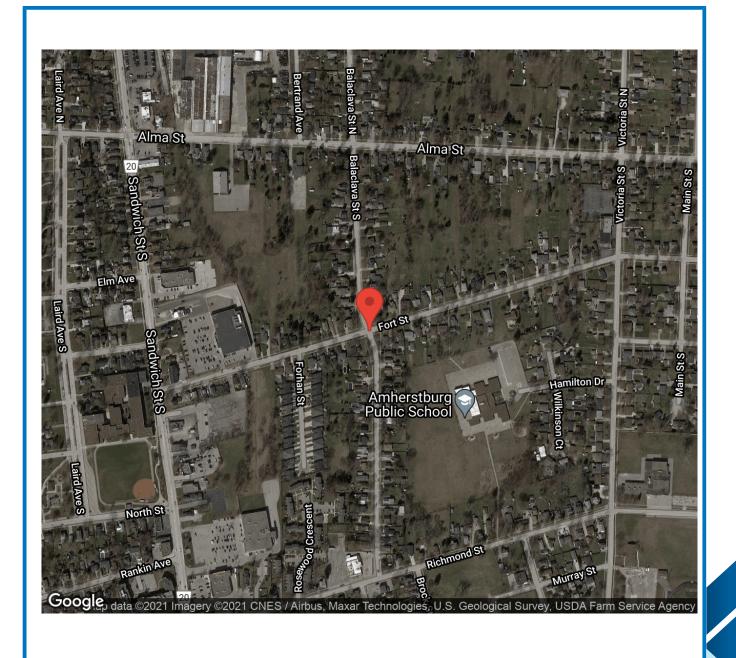


Traffic Count Map

Intersection: Balaclava St S & Fort St

Site Code: 2107800021 Municipality: Amherstburg

Count Date: Jun 16, 2021





Traffic Count Summary

Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

Balaclava St S - Traffic Summary

		North	Appr	oach T	otals			South	Appr	oach T	otals		
		Include	s Cars, 1	Trucks, Bi	cycles			Include	s Cars, 1	Γrucks, Bi	cycles		
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	2	9	1	0	12	1	1	14	0	0	15	3	27
08:00 - 09:00	3	11	6	0	20	0	2	16	3	0	21	3	4
					ВІ	REAK							
11:00 - 12:00	5	11	11	0	27	1	6	22	8	0	36	2	63
12:00 - 13:00	0	25	10	0	35	0	6	12	3	0	21	3	56
13:00 - 14:00	3	20	12	0	35	1	7	20	6	0	33	3	68
					ВІ	REAK						·	
15:00 - 16:00	7	37	8	0	52	1	4	26	6	0	36	3	88
16:00 - 17:00	3	36	13	1	53	1	12	20	2	0	34	0	87
17:00 - 18:00	2	13	21	0	36	0	14	14	4	0	32	2	68
GRAND TOTAL	25	162	82	1	270	5	52	144	32	0	228	19	498



Traffic Count Summary

Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

Fort St - Traffic Summary

		East	Appro	ach To	tals			West	Appro	oach To	otals		
		Include	s Cars, 1	Γrucks, Bi	cycles			Include	s Cars, 1	rucks, Bi	cycles		
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	3	11	4	0	18	1	4	4	4	0	12	0	30
08:00 - 09:00	5	17	2	0	24	0	6	18	7	0	31	2	55
					В	REAK						·	
11:00 - 12:00	6	24	0	0	30	3	6	38	16	0	60	1	90
12:00 - 13:00	5	29	1	0	35	3	13	43	19	0	75	0	110
13:00 - 14:00	6	19	2	0	27	3	14	38	19	0	71	1	98
					В	REAK						·	
15:00 - 16:00	5	33	4	0	42	1	17	51	20	0	88	2	130
16:00 - 17:00	8	36	5	0	49	2	9	53	23	0	85	3	134
17:00 - 18:00	4	21	7	0	32	0	11	52	24	0	87	0	119
GRAND TOTAL	42	190	25	0	257	13	80	297	132	0	509	9	766



Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

North Approach - Balaclava St S

			Cars				Ti	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	1	1	0	0	2	0	0	0	0	0	0	0	0	0	0	0
07:15	0	4	0	0	4	0	0	0	0	0	0	0	0	0	0	1
07:30	1	2	0	0	3	0	0	0	0	0	0	0	0	0	0	0
07:45	0	2	1	0	3	0	0	0	0	0	0	0	0	0	0	0
08:00	0	1	2	0	3	0	0	0	0	0	0	0	0	0	0	0
08:15	2	6	2	0	10	0	0	0	0	0	0	0	0	0	0	0
08:30	1	1	1	0	3	0	0	0	0	0	0	0	0	0	0	0
08:45	0	3	1	0	4	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	5	20	7	0	32	0	0	0	0	0	0	0	0	0	0	1



Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

North Approach - Balaclava St S

			Cars				Tı	rucks				Bio	cycles			
Start Time	4	1	-	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
11:00	2	1	4	0	7	0	0	0	0	0	0	0	0	0	0	0
11:15	1	4	4	0	9	0	0	0	0	0	1	0	0	0	1	0
11:30	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	1
11:45	1	3	3	0	7	0	0	0	0	0	0	0	0	0	0	0
12:00	0	5	2	0	7	0	0	0	0	0	0	0	0	0	0	0
12:15	0	4	1	0	5	0	0	0	0	0	0	2	0	0	2	0
12:30	0	8	3	0	11	0	0	0	0	0	0	0	0	0	0	0
12:45	0	6	4	0	10	0	0	0	0	0	0	0	0	0	0	0
13:00	2	5	4	0	11	0	0	0	0	0	0	0	0	0	0	0
13:15	1	3	3	0	7	0	1	0	0	1	0	0	0	0	0	0
13:30	0	4	1	0	5	0	0	0	0	0	0	0	2	0	2	0
13:45	0	7	1	0	8	0	0	0	0	0	0	0	1	0	1	1
SUBTOTAL	7	53	30	0	90	0	1	0	0	1	1	2	3	0	6	2



Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

North Approach - Balaclava St S

			Cars				T	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	2	12	0	0	14	0	0	0	0	0	0	0	1	0	1	0
15:15	1	9	3	0	13	0	0	0	0	0	0	1	1	0	2	0
15:30	4	7	1	0	12	0	0	0	0	0	0	0	0	0	0	0
15:45	0	8	2	0	10	0	0	0	0	0	0	0	0	0	0	1
16:00	1	8	3	0	12	0	0	0	0	0	0	0	0	0	0	0
16:15	0	13	3	0	16	0	0	0	0	0	0	0	0	0	0	0
16:30	1	8	2	0	11	1	0	0	0	1	0	0	0	0	0	0
16:45	0	7	5	0	12	0	0	0	0	0	0	0	0	1	1	1
17:00	0	5	9	0	14	0	0	0	0	0	0	0	0	0	0	0
17:15	0	3	6	0	9	0	0	0	0	0	0	0	0	0	0	0
17:30	2	3	4	0	9	0	0	0	0	0	0	0	0	0	0	0
17:45	0	2	2	0	4	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	11	85	40	0	136	1	0	0	0	1	0	1	2	1	4	2
GRAND TOTAL	23	158	77	0	258	1	1	0	0	2	1	3	5	1	10	5



Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

South Approach - Balaclava St S

			Cars				Ti	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	J	Total	4	1	•	1	Total	Total Peds
07:00	1	2	0	0	3	0	0	0	0	0	0	0	0	0	0	1
07:15	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0
07:30	0	7	0	0	7	0	0	0	0	0	0	0	0	0	0	0
07:45	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	2
08:00	1	8	0	0	9	0	0	0	0	0	0	0	0	0	0	1
08:15	0	4	1	0	5	0	0	0	0	0	0	0	0	0	0	2
08:30	0	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0
08:45	0	4	0	0	4	0	0	0	0	0	1	0	0	0	1	0
SUBTOTAL	2	30	3	0	35	0	0	0	0	0	1	0	0	0	1	6



Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

South Approach - Balaclava St S

			Cars				Ti	rucks				Bio	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
11:00	1	4	2	0	7	0	0	0	0	0	0	1	0	0	1	1
11:15	1	2	2	0	5	1	0	0	0	1	0	0	0	0	0	0
11:30	1	5	0	0	6	0	0	0	0	0	0	0	0	0	0	1
11:45	2	9	4	0	15	0	1	0	0	1	0	0	0	0	0	0
12:00	5	3	2	0	10	0	0	0	0	0	0	0	0	0	0	0
12:15	0	4	1	0	5	0	0	0	0	0	0	0	0	0	0	0
12:30	1	2	0	0	3	0	0	0	0	0	0	0	0	0	0	1
12:45	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	2
13:00	0	7	4	0	11	0	0	0	0	0	0	0	0	0	0	2
13:15	1	3	0	0	4	0	0	0	0	0	0	0	0	0	0	0
13:30	1	7	2	0	10	0	0	0	0	0	0	0	0	0	0	0
13:45	5	3	0	0	8	0	0	0	0	0	0	0	0	0	0	1
SUBTOTAL	18	52	17	0	87	1	1	0	0	2	0	1	0	0	1	8



Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

South Approach - Balaclava St S

			Cars				Ti	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	1	6	0	0	7	0	0	0	0	0	0	0	0	0	0	0
15:15	1	10	2	0	13	0	1	0	0	1	0	0	0	0	0	1
15:30	0	3	2	0	5	0	1	0	0	1	0	0	0	0	0	2
15:45	2	5	2	0	9	0	0	0	0	0	0	0	0	0	0	0
16:00	4	5	1	0	10	1	0	0	0	1	2	0	0	0	2	0
16:15	2	6	1	0	9	0	0	0	0	0	0	0	0	0	0	0
16:30	1	4	0	0	5	0	0	0	0	0	0	0	0	0	0	0
16:45	2	5	0	0	7	0	0	0	0	0	0	0	0	0	0	0
17:00	4	8	2	0	14	0	0	0	0	0	0	0	0	0	0	1
17:15	2	1	0	0	3	0	0	0	0	0	0	0	0	0	0	0
17:30	3	2	1	0	6	0	0	0	0	0	0	0	0	0	0	1
17:45	5	3	1	0	9	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	27	58	12	0	97	1	2	0	0	3	2	0	0	0	2	5
GRAND TOTAL	47	140	32	0	219	2	3	0	0	5	3	1	0	0	4	19



Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

East Approach - Fort St

			Cars				Tı	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	1	0	1	0	2	0	0	0	0	0	0	0	0	0	0	0
07:15	0	5	0	0	5	0	0	0	0	0	0	0	0	0	0	0
07:30	1	1	1	0	3	0	0	1	0	1	0	0	0	0	0	1
07:45	1	5	1	0	7	0	0	0	0	0	0	0	0	0	0	0
08:00	1	7	0	0	8	0	0	0	0	0	0	0	0	0	0	0
08:15	2	7	0	0	9	0	1	0	0	1	0	0	0	0	0	0
08:30	0	1	1	0	2	0	0	0	0	0	0	0	0	0	0	0
08:45	2	1	1	0	4	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	8	27	5	0	40	0	1	1	0	2	0	0	0	0	0	1



Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

East Approach - Fort St

			Cars				T	rucks				Bi	icycles			
Start Time	4	1	P	1	Total	4	1	P	1	Total	4	1	P	1	Total	Total Peds
11:00	1	7	0	0	8	0	0	0	0	0	0	0	0	0	0	3
11:15	2	7	0	0	9	0	0	0	0	0	0	0	0	0	0	0
11:30	2	6	0	0	8	0	0	0	0	0	0	0	0	0	0	0
11:45	1	4	0	0	5	0	0	0	0	0	0	0	0	0	0	0
12:00	1	5	0	0	6	0	0	0	0	0	0	0	0	0	0	0
12:15	1	9	0	0	10	0	0	0	0	0	0	0	0	0	0	0
12:30	1	9	1	0	11	0	0	0	0	0	0	0	0	0	0	1
12:45	2	5	0	0	7	0	0	0	0	0	0	1	0	0	1	2
13:00	0	3	0	0	3	0	1	0	0	1	0	0	0	0	0	0
13:15	2	7	0	0	9	0	0	0	0	0	0	0	0	0	0	2
13:30	1	2	2	0	5	0	0	0	0	0	0	0	0	0	0	0
13:45	3	6	0	0	9	0	0	0	0	0	0	0	0	0	0	1
SUBTOTAL	17	70	3	0	90	0	1	0	0	1	0	1	0	0	1	9



Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

East Approach - Fort St

		(Cars				T	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	2	8	1	0	11	0	0	0	0	0	0	4	0	0	4	0
15:15	0	6	1	0	7	1	0	0	0	1	0	0	1	0	1	1
15:30	1	3	0	0	4	0	0	0	0	0	0	4	0	0	4	0
15:45	1	8	1	0	10	0	0	0	0	0	0	0	0	0	0	0
16:00	2	11	1	0	14	0	0	0	0	0	1	0	0	0	1	0
16:15	1	9	2	0	12	0	0	0	0	0	0	0	0	0	0	1
16:30	3	12	2	0	17	0	0	0	0	0	0	0	0	0	0	0
16:45	1	4	0	0	5	0	0	0	0	0	0	0	0	0	0	1
17:00	1	5	2	0	8	0	0	0	0	0	0	0	0	0	0	0
17:15	1	4	0	0	5	0	0	0	0	0	0	0	1	0	1	0
17:30	1	5	1	0	7	0	0	0	0	0	0	0	0	0	0	0
17:45	1	7	2	0	10	0	0	0	0	0	0	0	1	0	1	0
SUBTOTAL	15	82	13	0	110	1	0	0	0	1	1	8	3	0	12	3
GRAND TOTAL	40	179	21	0	240	1	2	1	0	4	1	9	3	0	13	13



Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

West Approach - Fort St

Cars							Ti	rucks				Bio	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	1	1	0	0	2	0	0	0	0	0	0	0	0	0	0	0
07:15	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0
07:30	1	2	0	0	3	0	0	0	0	0	0	0	0	0	0	0
07:45	0	1	4	0	5	0	0	0	0	0	0	0	0	0	0	0
08:00	1	5	2	0	8	0	0	0	0	0	0	0	0	0	0	0
08:15	1	4	3	0	8	0	0	0	0	0	0	0	0	0	0	2
08:30	0	3	1	0	4	0	1	0	0	1	0	0	0	0	0	0
08:45	4	5	1	0	10	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	10	21	11	0	42	0	1	0	0	1	0	0	0	0	0	2



Traffic Count Data

Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

West Approach - Fort St

			Cars				Tr	rucks			Bicycles					
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
11:00	0	12	8	0	20	0	0	0	0	0	0	0	0	0	0	0
11:15	2	4	1	0	7	0	0	0	0	0	0	0	0	0	0	0
11:30	1	11	3	0	15	0	0	0	0	0	0	0	0	0	0	1
11:45	3	11	4	0	18	0	0	0	0	0	0	0	0	0	0	0
12:00	3	12	5	0	20	0	0	0	0	0	0	1	0	0	1	0
12:15	3	11	5	0	19	0	0	0	0	0	0	0	0	0	0	0
12:30	5	8	3	0	16	0	0	0	0	0	0	0	0	0	0	0
12:45	2	11	6	0	19	0	0	0	0	0	0	0	0	0	0	0
13:00	7	9	7	0	23	0	1	0	0	1	0	0	0	0	0	0
13:15	3	8	5	0	16	0	0	0	0	0	0	0	0	0	0	1
13:30	4	11	2	0	17	0	0	0	0	0	0	0	0	0	0	0
13:45	0	9	5	0	14	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	33	117	54	0	204	0	1	0	0	1	0	1	0	0	1	2



Traffic Count Data

Intersection: Balaclava St S & Fort St

Site Code: 2107800021

Municipality: Amherstburg

Count Date: Jun 16, 2021

West Approach - Fort St

			Cars				T	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	4	16	6	0	26	0	0	0	0	0	0	0	0	0	0	0
15:15	8	11	0	0	19	0	0	0	0	0	0	0	0	0	0	0
15:30	0	11	9	0	20	0	0	0	0	0	0	2	0	0	2	0
15:45	5	11	5	0	21	0	0	0	0	0	0	0	0	0	0	2
16:00	2	16	3	0	21	0	0	0	0	0	0	0	1	0	1	1
16:15	4	8	9	0	21	0	0	0	0	0	0	1	0	0	1	2
16:30	2	16	2	0	20	0	0	0	0	0	0	0	0	0	0	0
16:45	1	12	8	0	21	0	0	0	0	0	0	0	0	0	0	0
17:00	4	12	6	0	22	0	0	0	0	0	0	0	1	0	1	0
17:15	2	14	5	0	21	0	0	0	0	0	0	3	0	0	3	0
17:30	5	11	6	0	22	0	0	0	0	0	0	0	0	0	0	0
17:45	0	12	6	0	18	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	37	150	65	0	252	0	0	0	0	0	0	6	2	0	8	5
GRAND TOTAL	80	288	130	0	498	0	2	0	0	2	0	7	2	0	9	9



Peak Hour Diagram

Specified Period

One Hour Peak

To:

From:

To:

07:00:00 09:00:00

From: 08:00:00

09:00:00

Intersection:

Balaclava St S & Fort St

Site Code: Count Date: 2107800021 Jun 16, 2021 Weather conditions:

Clear

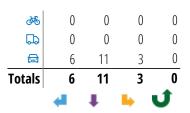
** Unsignalized Intersection **

Major Road: Fort St runs E/W

North Approach

	Out	In	Total
	20	24	44
	0	0	0
<i>₫</i> 6	0	0	0
	20	24	44

Balaclava St S



East Approach

	Out	In	Total
	23	23	46
	1	1	2
₫ %	0	0	0
	24	24	48

Fort St

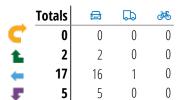
	Totals			₫	
7	0	0	0	0	
4	6	6	0	0	
\rightarrow	18	17	1	0	
4	7	7	0	0	



Peds: 2



Peds: 0



Fort St

Peds: 3

	Out	In	Total
	30	23	53
	1	1	2
<i>₹</i>	0	1	1
	31	25	56

West Approach

	4	1		T.
Totals	2	16	3	0
	1	16	3	0
₽	0	0	0	0
<i>₫</i> €	1	0	0	0

Balaclava St S

South Approach

	Out	In	Total
	20	23	43
	0	0	0
<i>₫</i>	1	0	1
	21	23	44







Comments



Peak Hour Summary

Intersection: Balaclava St S & Fort St

 Site Code:
 2107800021

 Count Date:
 Jun 16, 2021

Period: 07:00 - 09:00

Peak Hour Data (08:00 - 09:00)

		ľ	North A Balacla	pproac ıva St S	h			S	outh A Balacla	pproac ava St S	h				East A _l Foi	oproach rt St	1			Ī	West Ap For	proacl t St	h		Total Vehicl
Start Time	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	es
08:00	0	1	2	0	0	3	1	8	0	0	1	9	1	7	0	0	0	8	1	5	2	0	0	8	28
08:15	2	6	2	0	0	10	0	4	1	0	2	5	2	8	0	0	0	10	1	4	3	0	2	8	33
08:30	1	1	1	0	0	3	0	0	2	0	0	2	0	1	1	0	0	2	0	4	1	0	0	5	12
08:45	0	3	1	0	0	4	1	4	0	0	0	5	2	1	1	0	0	4	4	5	1	0	0	10	23
Grand Total	3	11	6	0	0	20	2	16	3	0	3	21	5	17	2	0	0	24	6	18	7	0	2	31	96
Approach %	15	55	30	0		-	9.5	76.2	14.3	0		-	20.8	70.8	8.3	0		-	19.4	58.1	22.6	0		-	
Totals %	3.1	11.5	6.3	0		20.8	2.1	16.7	3.1	0	,	21.9	5.2	17.7	2.1	0	,	25	6.3	18.8	7.3	0		32.3	
PHF	0.38	0.46	0.75	0		0.5	0.5	0.5	0.38	0		0.58	0.63	0.53	0.5	0		0.6	0.38	0.9	0.58	0		0.78	0.73
Cars	3	11	6	0		20	1	16	3	0		20	5	16	2	0		23	6	17	7	0		30	93
% Cars	100	100	100	0		100	50	100	100	0		95.2	100	94.1	100	0		95.8	100	94.4	100	0		96.8	96.9
Trucks	0	0	0	0		0	0	0	0	0		0	0	1	0	0		1	0	1	0	0		1	2
% Trucks	0	0	0	0		0	0	0	0	0		0	0	5.9	0	0		4.2	0	5.6	0	0		3.2	2.1
Bicycles	0	0	0	0		0	1	0	0	0		1	0	0	0	0		0	0	0	0	0		0	1
% Bicycles	0	0	0	0		0	50	0	0	0		4.8	0	0	0	0		0	0	0	0	0		0	1
Peds					0	-					3	-					0	-					2	-	5
% Peds					0	-					60	-					0	-					40	-	



Peak Hour Diagram

Specified Period

One Hour Peak

From: 11:00:00 To: 14:00:00

From: 11:45:00 To: 12:45:00

Intersection: Balaclava St S & Fort St

 Site Code:
 2107800021

 Count Date:
 Jun 16, 2021

Weather conditions:

Clear

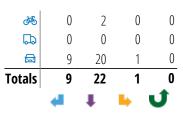
** Unsignalized Intersection **

Major Road: Fort St runs E/W

North Approach

	Out	In	Total
	30	33	63
	0	1	1
ॐ	2	0	2
,	32	34	66

Balaclava St S



East Approach

	Out	In	Total
	32	50	82
	0	0	0
₩	0	1	1
	32	51	83

Fort St

	Totals			<i>₫</i> %	
7	0	0	0	0	
4	14	14	0	0	
\Rightarrow	43	42	0	1	
4	17	17	0	0	





Fort St

	Totals			<i>₫</i> 6
C	0	0	0	0
£	1	1	0	0
-	27	27	0	0
F	4	4	0	0

West Approach

	Out	In	Total
	73	44	117
	0	0	0
<i>₹</i>	1	0	1
	74	44	118

	4	1		.1
Totals	8	19	7	0
	8	18	7	0
₽	0	1	0	0
₫%	0	0	0	0

Peds: 1

Balaclava St S

South Approach

	Out	ln	Total
	33	41	74
	1	0	1
₫	0	2	2
,	34	43	77







Comments



Peak Hour Summary

Intersection: Balaclava St S & Fort St

 Site Code:
 2107800021

 Count Date:
 Jun 16, 2021

 Period:
 11:00 - 14:00

Peak Hour Data (11:45 - 12:45)

		ľ	North <i>A</i> Balack	Approac ava St S	h			S	outh A Balacla	pproac ava St S	h					pproach rt St	1			1	West A For	pproacl t St	h		Total Vehicl
Start Time	4	1		J	Peds	Total	4	1	•	J	Peds	Total	4	1		J	Peds	Total	4	1	•	J	Peds	Total	es
11:45	1	3	3	0	0	7	2	10	4	0	0	16	1	4	0	0	0	5	3	11	4	0	0	18	46
12:00	0	5	2	0	0	7	5	3	2	0	0	10	1	5	0	0	0	6	3	13	5	0	0	21	44
12:15	0	6	1	0	0	7	0	4	1	0	0	5	1	9	0	0	0	10	3	11	5	0	0	19	41
12:30	0	8	3	0	0	11	1	2	0	0	1	3	1	9	1	0	1	11	5	8	3	0	0	16	41
Grand Total	1	22	9	0	0	32	8	19	7	0	1	34	4	27	1	0	1	32	14	43	17	0	0	74	172
Approach %	3.1	68.8	28.1	0		-	23.5	55.9	20.6	0		-	12.5	84.4	3.1	0		-	18.9	58.1	23	0		-	
Totals %	0.6	12.8	5.2	0	,	18.6	4.7	11	4.1	0		19.8	2.3	15.7	0.6	0	,	18.6	8.1	25	9.9	0		43	
PHF	0.25	0.69	0.75	0		0.73	0.4	0.48	0.44	0		0.53	1	0.75	0.25	0		0.73	0.7	0.83	0.85	0		0.88	0.93
Cars	1	20	9	0		30	8	18	7	0		33	4	27	1	0		32	14	42	17	0		73	168
% Cars	100	90.9	100	0		93.8	100	94.7	100	0		97.1	100	100	100	0		100	100	97.7	100	0		98.6	97.7
Trucks	0	0	0	0		0	0	1	0	0		1	0	0	0	0		0	0	0	0	0		0	1
% Trucks	0	0	0	0		0	0	5.3	0	0		2.9	0	0	0	0		0	0	0	0	0		0	0.6
Bicycles	0	2	0	0		2	0	0	0	0		0	0	0	0	0		0	0	1	0	0		1	3
% Bicycles	0	9.1	0	0		6.3	0	0	0	0		0	0	0	0	0		0	0	2.3	0	0		1.4	1.7
Peds					0	-					1	-					1	-					0	-	2
% Peds					0	-					50	-					50	-					0	-	



Peak Hour Diagram

Specified Period

One Hour Peak

From: 15:00:00 To: 18:00:00

From: 15:45:00 To: 16:45:00

Intersection: Balaclava St S & Fort St

 Site Code:
 2107800021

 Count Date:
 Jun 16, 2021

Weather conditions:

Clear

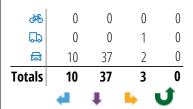
** Unsignalized Intersection **

Major Road: Fort St runs E/W

North Approach

	Out	In	Total
	49	39	88
	1	0	1
<i>₫</i>	0	0	0
	50	39	89

Balaclava St S



East Approach

	Out	In	Total
	53	57	110
	0	1	1
ॐ	1	1	2
	54	59	113

Fort St

	Totals			₫ ®	
7	0	0	0	0	
4	13	13	0	0	
\Rightarrow	52	51	0	1	
4	20	19	0	1	

Peds: 1



Fort St

	Totals	⊟		<i>₫</i>
C	0	0	0	0
£	6	6	0	0
-	40	40	0	0
F	8	7	0	1

West Approach

	Out	In	Total
	83	59	142
	0	1	1
<i>₫</i>	2	2	4
	85	62	147

	4	1		J
Totals	12	20	4	0
	9	20	4	0
	1	0	0	0

Peds: 0

Balaclava St S

South Approach

	Out	ln	Total
	33	63	96
	1	0	1
<i>₩</i>	2	2	4
	36	65	101







0

Comments



Peak Hour Summary

Intersection: Balaclava St S & Fort St

 Site Code:
 2107800021

 Count Date:
 Jun 16, 2021

Period: 15:00 - 18:00

Peak Hour Data (15:45 - 16:45)

		ľ	North A Balacla	pproac ava St S	:h			S	outh <i>A</i> Balack	Approac ava St S	h					pproacl rt St	h			Ī	West A _l For	pproacl t St	h		Total Vehicl
Start Time	4	1	•	J	Peds	Total	4	1	P	J	Peds	Total	4	1	P	J	Peds	Total	4	1	•	J	Peds	Total	es
15:45	0	8	2	0	1	10	2	5	2	0	0	9	1	8	1	0	0	10	5	11	5	0	2	21	50
16:00	1	8	3	0	0	12	7	5	1	0	0	13	3	11	1	0	0	15	2	16	4	0	1	22	62
16:15	0	13	3	0	0	16	2	6	1	0	0	9	1	9	2	0	1	12	4	9	9	0	2	22	59
16:30	2	8	2	0	0	12	1	4	0	0	0	5	3	12	2	0	0	17	2	16	2	0	0	20	54
Grand Total	3	37	10	0	1	50	12	20	4	0	0	36	8	40	6	0	1	54	13	52	20	0	5	85	225
Approach %	6	74	20	0		-	33.3	55.6	11.1	0		-	14.8	74.1	11.1	0		-	15.3	61.2	23.5	0		-	
Totals %	1.3	16.4	4.4	0		22.2	5.3	8.9	1.8	0	,	16	3.6	17.8	2.7	0		24	5.8	23.1	8.9	0	,	37.8	
PHF	0.38	0.71	0.83	0		0.78	0.43	0.83	0.5	0		0.69	0.67	0.83	0.75	0		0.79	0.65	0.81	0.56	0		0.97	0.91
Cars	2	37	10	0		49	9	20	4	0		33	7	40	6	0		53	13	51	19	0		83	218
% Cars	66.7	100	100	0		98	75	100	100	0		91.7	87.5	100	100	0		98.1	100	98.1	95	0		97.6	96.9
Trucks	1	0	0	0		1	1	0	0	0		1	0	0	0	0		0	0	0	0	0		0	2
% Trucks	33.3	0	0	0		2	8.3	0	0	0		2.8	0	0	0	0		0	0	0	0	0		0	0.9
Bicycles	0	0	0	0		0	2	0	0	0		2	1	0	0	0		1	0	1	1	0		2	5
% Bicycles	0	0	0	0		0	16.7	0	0	0		5.6	12.5	0	0	0		1.9	0	1.9	5	0		2.4	2.2
Peds					1	-					0	-					1	-					5	-	7
% Peds					14.3	-					0	-					14.3	-					71.4	-	



From: Robin Marinac

Sent: January 13, 2022 11:49 AM To: fgarardo@amherstburg.ca

Subject: Traffic Data Request - Sandwich Street TIS and Parking Study

Hi Frank,

I am a traffic consultant looking to obtain traffic and transportation data for use in a TIS for 131 Sandwich Street – a site I believe you are familiar with. I apologize if you are not the correct person to contact for this information and ask that you please forward my request to the appropriate individual.

Could you please provide the following data if available:

- TMC at Sandwich Street South and Fort Street We do not track this
- TMC at North Street and Sandwich Street South We do not track this
- TMC at Sandwich Street South and the parking lot access shown in the figure below We do not track this
- STP for the Signalized Pedestrian Crossing on Sandwich Street, approximately 75 metres south of Fort Street Unfamiliar with STP terminology
- Any ATR/AADT counts along Sandwich Street between Maple Avenue and Rankin Avenue Refer to County of Essex mapping data for Sandwich Street information (if any)
- Any ATR/AADT counts along Fort Street between Sandwich Street and Balaclava Street 1400 2016 (2021 counts skewed by pandemic)
- Any ATR/AADT counts along North Street between Sandwich Street and Dalhousie Street 700 2016 (2021 counts skewed by pandemic)
- Any collision data (preferably over the past 5 years) at the intersections of Sandwich Street at Fort Street and Sandwich Street and North Street. Do not track
- Any collision data (preferably over the past 5 years) along Sandwich Street between Maple Avenue and Rankin Avenue Do not track
- Any collision data (preferably over the past 5 years) along Fort Street between Sandwich Street and Balaclava Street Do not track
- Any collision data (preferably over the past 5 years) along North Street between Sandwich Street and Dalhousie
 Street Do not track

Thank you very much in advance for your help.

Kind regards, Robin Marinac



Robin Marinac, EIT CGH Transportation Inc. P: 437-242-5183

E: robin.marinac@cghtransportation.com

Appendix D

Heavy Vehicle Percentage Calculations

	[1] Sandwich Street S & Fort Street											
	АМ											
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
HV Volume	0	11	1	0	19	0	0	0	0	1	0	0
Total Volume	0	335	46	20	454	0	0	0	0	12	0	15
HV%	-	3%	2%	0%	4%	-	-	-	-	8%	-	0%
						PM						
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
HV Volume	0	8	0	0	3	0	0	0	0	0	0	0
Total Volume	0	550	68	12	563	0	0	0	0	36	0	21
HV%	-	1%	0%	0%	1%	-	-	-	-	0%	-	0%

				[2] Sand	dwich Stree	et S & Parkin	g Lot Drivev	vay				
						AM						
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WB
HV Volume	0	12	0	0	20	0	0	0	0	0	0	0
Total Volume	0	362	27	13	453	0	0	0	0	1	0	19
HV%	-	3%	0%	0%	4%	-	-	-	-	0%	-	0%
						PM						
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WB
HV Volume	0	8	0	0	3	0	0	0	0	0	0	0
Total Volume	0	616	5	0	599	0	0	0	0	3	0	2
HV%	-	1%	0%	#DIV/0!	1%	-	-	-	-	0%	-	0%

			[3] 9	Sandwich St	reet S & No	rth Street /	165 Sandw	ich Street S				
						AM						
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
HV Volume	4	11	0	0	20	1	0	0	0	0	0	0
Total Volume	53	382	4	5	432	23	2	0	15	1	0	0
HV%	8%	3%	0%	0%	5%	4%	0%	-	0%	0%	-	-
						PM						
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
HV Volume	0	8	0	0	3	0	0	0	0	0	0	0
Total Volume	3	615	0	2	594	6	3	0	4	3	0	3
HV%	0%	1%	-	0%	1%	0%	0%	-	0%	0%	-	0%

Appendix E

2022 Existing Synchro & SimTraffic Worksheets

Lanes, Volumes, Timings 1: Sandwich Street South & Fort Street

	•	4	†	~	>	ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	7	^		ሻ	†
Traffic Volume (vph)	14	18	386	53	23	523
Future Volume (vph)	14	18	386	53	23	523
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	20.0	0.0		0.0	30.0	
Storage Lanes	1	1		0	1	
Taper Length (m)	15.0				15.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt		0.850	0.984			
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1653	1566	1797	0	1750	1807
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1653	1566	1797	0	1750	1807
Link Speed (k/h)	40		50			50
Link Distance (m)	238.1		44.2			71.5
Travel Time (s)	21.4		3.2			5.1
Confl. Peds. (#/hr)		1		5	5	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	8%	2%	3%	2%	2%	4%
Adj. Flow (vph)	15	19	406	56	24	551
Shared Lane Traffic (%)						
Lane Group Flow (vph)	15	19	462	0	24	551
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		3.5			3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	15		15	25	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	tion 37.9%			IC	CU Level o	of Service A

Analysis Period (min) 15

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HCM 95th %tile Q(veh)

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	7	î,		ች	†
Traffic Vol, veh/h	14	18	386	53	23	523
Future Vol, veh/h	14	18	386	53	23	523
Conflicting Peds, #/hr	0	1	0	5	5	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	_		-	None	-	None
Storage Length	200	0	-	-	300	-
Veh in Median Storage	, # 0	-	0	_	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	8	2	3	2	2	4
Mvmt Flow	15	19	406	56	24	551
Major/Minor I	Minor1	N	/lajor1		Major2	
Conflicting Flow All	1038	440	0	0	467	0
Stage 1	439	-	-	-	- -	-
Stage 2	599	_	_	_	_	_
Critical Hdwy	6.48	6.22	_	_	4.12	_
Critical Hdwy Stg 1	5.48	0.22	_	_	7.12	<u>-</u>
Critical Hdwy Stg 2	5.48	_	_	_	_	_
Follow-up Hdwy	3.572	3 318	_	_	2.218	_
Pot Cap-1 Maneuver	249	617	_	_	1094	_
Stage 1	637	-	_	_	-	_
Stage 2	537	_	_	_	_	_
Platoon blocked, %	001		_	_		_
Mov Cap-1 Maneuver	242	614	_	_	1089	_
Mov Cap-2 Maneuver	371	-	_	_	-	_
Stage 1	634	_	_	_	_	_
Stage 2	525	_	_	_	_	_
Olago Z	020					
	14/5		ND		0.0	
Approach	WB		NB		SB	
HCM Control Delay, s	12.8		0		0.4	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1V	VBLn2	SBL
Capacity (veh/h)		_	_	371	614	1089
HCM Lane V/C Ratio		-	_		0.031	
HCM Control Delay (s)		_	_		11.1	8.4
HCM Lane LOS		-	-	С	В	Α
LICM OF the O/ tile O/ cole				0.4	0.4	0.4

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Lanes, Volumes, Timings 2: Sandwich Street South & Parking Lot Driveway

	•	•	†	/	>	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1>		ሻ	†
Traffic Volume (vph)	2	22	417	32	15	521
Future Volume (vph)	2	22	417	32	15	521
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0		0.0	15.0	
Storage Lanes	1	0		0	1	
Taper Length (m)	15.0				15.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.876		0.990			
Flt Protected	0.996				0.950	
Satd. Flow (prot)	1607	0	1807	0	1750	1807
Flt Permitted	0.996				0.950	
Satd. Flow (perm)	1607	0	1807	0	1750	1807
Link Speed (k/h)	30		50			50
Link Distance (m)	133.6		37.5			44.2
Travel Time (s)	16.0		2.7			3.2
Confl. Peds. (#/hr)				19	19	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	2%	2%	3%	2%	2%	4%
Adj. Flow (vph)	2	23	434	33	16	543
Shared Lane Traffic (%)						
Lane Group Flow (vph)	25	0	467	0	16	543
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5	J -	3.5	<u> </u>		3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	15		15	25	
Sign Control	Stop	• •	Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizati	ion 37.4%			IC	CU Level c	of Service

Analysis Period (min) 15

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Intersection						
Int Delay, s/veh	0.4					
	WDI	WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĵ.		ች	↑
Traffic Vol, veh/h	2	22	417	32	15	521
Future Vol, veh/h	2	22	417	32	15	521
Conflicting Peds, #/hr	0	0	0	19	19	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	_	0	-	-	0
Peak Hour Factor	96	96	96	96	96	96
Heavy Vehicles, %	2	2	3	2	2	4
Mymt Flow	2	23	434	33	16	543
IVIVIII(I IOW	2	20	707	55	10	J - 10
Major/Minor	Minor1	N	//ajor1	ı	Major2	
Conflicting Flow All	1045	470	0	0	486	0
Stage 1	470	_	_	_	_	_
Stage 2	575	_	_	_	_	_
Critical Hdwy	6.42	6.22	_	_	4.12	_
Critical Hdwy Stg 1	5.42	- 0.22	_		7.12	_
	5.42	_	_	_	_	_
Critical Hdwy Stg 2			-	_	2.218	
Follow-up Hdwy	3.518		-			-
Pot Cap-1 Maneuver	253	594	-	-	1077	-
Stage 1	629	-	-	-	-	-
Stage 2	563	-	-	-	-	-
Platoon blocked, %			-			-
Mov Cap-1 Maneuver	245	583	-	-	1058	-
Mov Cap-2 Maneuver	378	-	-	-	-	-
Stage 1	618	-	-	-	-	-
Stage 2	555	-	_	_	_	_
	300					
Approach	WB		NB		SB	
HCM Control Delay, s	11.8		0		0.2	
HCM LOS	В					
Minor Long (Maior M	.1	NDT	MDDV	VDI 4	CDI	CDT
Minor Lane/Major Mvn	π	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-	-	558	1058	-
HCM Lane V/C Ratio		-	-	0.045		-
HCM Control Delay (s)		-	-	11.8	8.5	-
HCM Lane LOS		-	-	В	Α	-
HCM 95th %tile Q(veh)	-	-	0.1	0	-
	,					

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3: Sandwich Street South & North Street/165 Sandwich Street South Drive was Sandwich Street South

	٠	→	•	•	←	•	4	†	~	/	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	ĵ.		ሻ	f)	
Traffic Volume (vph)	3	0	18	2	0	0	61	440	5	6	497	27
Future Volume (vph)	3	0	18	2	0	0	61	440	5	6	497	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0		0.0	0.0		0.0	10.0		0.0	30.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.883						0.998			0.992	
Flt Protected		0.994			0.950		0.950			0.950		
Satd. Flow (prot)	0	1455	0	0	1750	0	1653	1821	0	1750	1776	0
Flt Permitted		0.994			0.950		0.950			0.950		
Satd. Flow (perm)	0	1455	0	0	1750	0	1653	1821	0	1750	1776	0
Link Speed (k/h)		50			30			50			50	
Link Distance (m)		127.1			58.9			86.3			121.9	
Travel Time (s)		9.2			7.1			6.2			8.8	
Confl. Peds. (#/hr)			1	1			9		26	26		9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	8%	3%	2%	2%	5%	4%
Parking (#/hr)		0										
Adj. Flow (vph)	3	0	20	2	0	0	68	489	6	7	552	30
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	23	0	0	2	0	68	495	0	7	582	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.01	1.16	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
	Other											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 44.9%			10	CU Level	of Service	Α					

Intersection Capacity Utilization 44.9%

ICU Level of Service A

Analysis Period (min) 15

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3: Sandwich Street South & North Street/165 Sandwich Street South Drive was Sandwich Street South

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ች	ĵ.			ĵ.	
Traffic Vol, veh/h	3	0	18	2	0	0	61	440	5	6	497	27
Future Vol, veh/h	3	0	18	2	0	0	61	440	5	6	497	27
Conflicting Peds, #/hr	0	0	1	1	0	0	9	0	26	26	0	9
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	_	-	None	-	-	None	_	_	None	_	_	None
Storage Length	_	_	-	-	_	-	100	_	-	300	-	-
Veh in Median Storage	e.# -	0	-	-	0	-	_	0	-	-	0	_
Grade, %	-,	0	-	-	0	-	_	0	_	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	8	3	2	2	5	4
Mvmt Flow	3	0	20	2	0	0	68	489	6	7	552	30
								.00		•		
Major/Minor	Minor2			Minor1			Major1		N	Major2		
Conflicting Flow All	1218	1247	577	1246	1259	518	591	0	0	521	0	0
Stage 1	590	590	-	654	654	-	-	-	-	-	-	-
Stage 2	628	657	_	592	605	_	_	_	_	_	_	_
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.18	_	_	4.12	_	_
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	_	_	-	_	_
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	_	_	_	_	_	_	_
Follow-up Hdwy	3.518	4.018		3.518	4.018	3.318	2.272	_	_	2.218	_	_
Pot Cap-1 Maneuver	157	173	516	151	171	558	956	_	_		_	_
Stage 1	494	495	-	456	463	-	-	_	_	-	-	_
Stage 2	471	462	-	493	487	-	-	-	-	_	_	_
Platoon blocked, %								-	_		-	-
Mov Cap-1 Maneuver	146	154	511	133	153	544	948	_	-	1020	-	-
Mov Cap-2 Maneuver	146	154	-	133	153	-	-	_	_	-	-	-
Stage 1	454	488	-	413	419	_	_	-	_	_	-	-
Stage 2	437	419	_	470	480	_	_	_	_	_	-	_
0 -												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	15.2			32.5			1.1			0.1		
HCM LOS	С			D								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		948	-	-	377	133	1020	-	-			
HCM Lane V/C Ratio		0.071	-	-	0.062			-	-			
HCM Control Delay (s)		9.1	-	-	15.2	32.5	8.6	-	-			
HCM Lane LOS		Α	-	-	С	D	Α	-	-			
HCM 95th %tile Q(veh)	0.2	-	-	0.2	0.1	0	-	-			

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Lanes, Volumes, Timings 4: Sandwich Street South & Pedestrian Crossing

	۶	→	•	•	←	•	•	†	~	>	ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations								*			†	
Traffic Volume (vph)	0	0	0	0	0	0	0	449	0	0	523	0
Future Volume (vph)	0	0	0	0	0	0	0	449	0	0	523	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt												
Flt Protected												
Satd. Flow (prot)	0	0	0	0	0	0	0	1842	0	0	1842	0
Flt Permitted	•	-			•	-	•					-
Satd. Flow (perm)	0	0	0	0	0	0	0	1842	0	0	1842	0
Right Turn on Red	•	•	Yes			Yes			Yes	•		Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		27.0			26.6			121.9			37.5	
Travel Time (s)		1.9			1.9			8.8			2.7	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	468	0.00	0.00	545	0.00
Shared Lane Traffic (%)								100			0.10	
Lane Group Flow (vph)	0	0	0	0	0	0	0	468	0	0	545	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	Loit	0.0	rtigit	Loit	0.0	rtigitt	Loit	3.5	ragne	Loit	3.5	ragne
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane		0.0			0.0			Yes			Yes	
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	1.01	15	25	1.01	15	25	1.01	15	25	1.01	15
Number of Detectors	20		10	20		10	20	2	10	20	2	10
Detector Template								Thru			Thru	
Leading Detector (m)								10.0			10.0	
Trailing Detector (m)								0.0			0.0	
Detector 1 Position(m)								0.0			0.0	
Detector 1 Size(m)								0.6			0.6	
Detector 1 Type								CI+Ex			Cl+Ex	
Detector 1 Channel								OITEX			OITEX	
Detector 1 Extend (s)								0.0			0.0	
Detector 1 Queue (s)								0.0			0.0	
Detector 1 Delay (s)								0.0			0.0	
Detector 2 Position(m)								9.4			9.4	
Detector 2 Size(m)								0.6			0.6	
Detector 2 Type								Cl+Ex			CI+Ex	
Detector 2 Channel								OITEX			OITEX	
Detector 2 Extend (s)								0.0			0.0	
Turn Type								NA			NA	
Protected Phases								2			6	
Permitted Phases											U	
Detector Phase								2			6	
Switch Phase								۷			Ü	
Minimum Initial (s)								10.0			10.0	
willing things (5)								10.0			10.0	

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Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Peak Hour Factor	
Adj. Flow (vph) Shared Lane Traffic (%)	
. ,	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
•	

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	۶	→	•	•	—	•	1	†	/	/	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)								24.5			24.5	
Total Split (s)								30.0			30.0	
Total Split (%)								50.0%			50.0%	
Maximum Green (s)								23.5			23.5	
Yellow Time (s)								3.3			3.3	
All-Red Time (s)								3.2			3.2	
Lost Time Adjust (s)								0.0			0.0	
Total Lost Time (s)								6.5			6.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)								3.0			3.0	
Recall Mode								Max			Max	
Walk Time (s)								7.0			7.0	
Flash Dont Walk (s)								11.0			11.0	
Pedestrian Calls (#/hr)								0			0	
Act Effct Green (s)								42.1			42.1	
Actuated g/C Ratio								0.61			0.61	
v/c Ratio								0.42			0.48	
								12.5			13.5	
Control Delay								0.0			0.0	
Queue Delay								12.5			13.5	
Total Delay												
LOS								B 12.5			B	
Approach Delay											13.5	
Approach LOS								B			B	
Queue Length 50th (m)								40.5			49.9	
Queue Length 95th (m)		2.0			0.0			63.8			77.9	
Internal Link Dist (m)		3.0			2.6			97.9			13.5	
Turn Bay Length (m)								4404			4404	
Base Capacity (vph)								1124			1124	
Starvation Cap Reductn								0			0	
Spillback Cap Reductn								0			0	
Storage Cap Reductn								0			0	
Reduced v/c Ratio								0.42			0.48	
Intersection Summary												
<i>7</i> 1	ther											
Cycle Length: 60												
Actuated Cycle Length: 69												
Natural Cycle: 60												
Control Type: Semi Act-Unco	ord											
Maximum v/c Ratio: 0.48												
Intersection Signal Delay: 13.					tersection							
Intersection Capacity Utilization	on 32.9%			IC	U Level	of Service	A					
Analysis Period (min) 15												
Splits and Phases: 4: Sand	wich Stre	et South	& Pedest	rian Cros	sing							
†					Ă₽,							
Ø2						04						
30 s					30 s							
₩ Ø6												,
30 s												

Lane Group	Ø4
Minimum Split (s)	30.0
Total Split (s)	30.0
Total Split (%)	50%
Maximum Green (s)	26.0
Yellow Time (s)	3.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	19.0
Pedestrian Calls (#/hr)	137
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

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Lanes, Volumes, Timings 1: Sandwich Street South & Fort Street

	•	•	†	~	/	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	f _a		ሻ	†
Traffic Volume (vph)	42	25	633	79	14	648
Future Volume (vph)	42	25	633	79	14	648
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	20.0	0.0		0.0	30.0	
Storage Lanes	1	1		0	1	
Taper Length (m)	15.0				15.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt		0.850	0.985			
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1750	1566	1814	0	1750	1842
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1750	1566	1814	0	1750	1842
Link Speed (k/h)	40		50			50
Link Distance (m)	238.1		44.2			71.5
Travel Time (s)	21.4		3.2			5.1
Confl. Peds. (#/hr)	1			5	5	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	43	26	653	81	14	668
Shared Lane Traffic (%)						
Lane Group Flow (vph)	43	26	734	0	14	668
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		3.5			3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	15		15	25	
Sign Control	Stop		Free			Free
Intersection Summary						
· · · · · · · · · · · · · · · · · · ·	Other					
Control Type: Unsignalized	Ou lei					
	tion 49 20/			10	CU Level o	of Consider
Intersection Capacity Utilizat	uon 48.2%			IC	o Level (o Service
Analysis Period (min) 15						

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Intersection							
Int Delay, s/veh	0.9						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	YVDL	VVDI€	1\D1	אטוו	JDL T	<u> </u>	
Traffic Vol, veh/h	42	25	633	79	14	648	
Future Vol, veh/h	42	25	633	79	14	648	
Conflicting Peds, #/hr	1	0	033	5	5	040	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	Stop -	None	-	None	-	None	
Storage Length	200	0	-	NOITE	300	None -	
Veh in Median Storage		-	0	<u>-</u>	300	0	
	, # 0		0				
Grade, %		- 07		- 07	- 07	0	
Peak Hour Factor	97	97	97	97	97	97	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	43	26	653	81	14	668	
Major/Minor N	Minor1	N	//ajor1		Major2		
Conflicting Flow All	1396	699	0	0	739	0	
Stage 1	699	-	-	-	-	-	
Stage 2	697	_	_			_	
Critical Hdwy	6.42	6.22	-	_	4.12	_	
Critical Hdwy Stg 1	5.42	0.22	_	_	7.12	_	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
, ,	3.518	3.318	_	-	2.218	-	
Follow-up Hdwy		440		-	867		
Pot Cap-1 Maneuver	156	440	-	-	007	-	
Stage 1	493	-	-	-	-	-	
Stage 2	494	-	-	-	-	-	
Platoon blocked, %	4-0	400	-	-		-	
Mov Cap-1 Maneuver	153	438	-	-	863	-	
Mov Cap-2 Maneuver	293	-	-	-	-	-	
Stage 1	491	-	-	-	-	-	
Stage 2	486	-	-	-	-		
Approach	WB		NB		SB		
HCM Control Delay, s	17.3		0		0.2		
HCM LOS	C		U		0.2		
I IOIVI LOO	U						
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1V	VBLn2	SBL	
Capacity (veh/h)		-	-	293	438	863	
HCM Lane V/C Ratio		_				0.017	
		-	-	0.110			
HCM Control Delay (s)		-	_	19.4	13.7	9.2	
			- -			9.2 A	
						^ ^	

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Lanes, Volumes, Timings 2: Sandwich Street South & Parking Lot Driveway

	•	4	†	<i>></i>	\	 	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	¥		^		<u> </u>	<u> </u>	
Traffic Volume (vph)	4	3	709	6	0	689	
Future Volume (vph)	4	3	709	6	0	689	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (m)	0.0	0.0		0.0	15.0		
Storage Lanes	1	0		0	1		
Taper Length (m)	15.0				15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor							
Frt	0.942		0.999				
Flt Protected	0.972						
Satd. Flow (prot)	1687	0	1840	0	1842	1842	
Flt Permitted	0.972						
Satd. Flow (perm)	1687	0	1840	0	1842	1842	
Link Speed (k/h)	30		50			50	
Link Distance (m)	133.6		35.2			44.2	
Travel Time (s)	16.0		2.5			3.2	
Confl. Peds. (#/hr)	4	1		4	4		
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	
Adj. Flow (vph)	4	3	723	6	0	703	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	7	0	729	0	0	703	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Left	
Median Width(m)	3.5		3.5			3.5	
Link Offset(m)	0.0		0.0			0.0	
Crosswalk Width(m)	3.0		3.0			3.0	
Two way Left Turn Lane			Yes			Yes	
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01	
Turning Speed (k/h)	25	15		15	25		
Sign Control	Stop		Free			Free	
Intersection Summary							
	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	tion 48.0%			IC	U Level of	of Service /	Α
Analysis Period (min) 15							

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Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	WBL	WDN		NDI	SBL Š	
Lane Configurations		2	700	6		690
Traffic Vol, veh/h	4	3	709	6	0	689
Future Vol, veh/h	4	3	709	6	0	689
Conflicting Peds, #/hr	4	1	_ 0	_ 4	_ 4	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	3	723	6	0	703
	•		•		•	
		_				
Major/Minor	Minor1		//ajor1	N	Major2	
Conflicting Flow All	1437	731	0	0	733	0
Stage 1	730	-	-	-	-	-
Stage 2	707	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	_	-	_	_	-
Critical Hdwy Stg 2	5.42	_	_	_	_	_
Follow-up Hdwy	3.518	3 318	_	_	2.218	_
Pot Cap-1 Maneuver	147	422	_	_	872	_
Stage 1	477		_	_	-	_
Stage 2	489	-	-	_	_	_
	409	_	_		_	
Platoon blocked, %	4.40	100	-	-	000	-
Mov Cap-1 Maneuver	146	420	-	-	869	-
Mov Cap-2 Maneuver	286	-	-	-	-	-
Stage 1	475	-	-	-	-	-
Stage 2	487	-	-	-	-	-
Annroach	WB		NB		SB	
Approach						
HCM Control Delay, s	16.1		0		0	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NBT	NRRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-		869	
HCM Lane V/C Ratio				0.022		•
		-			-	-
HCM Control Delay (s)		-	-	16.1	0	-
HCM Lane LOS	\	-	-	C	A	-
HCM 95th %tile Q(veh)	-	-	0.1	0	-

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3: Sandwich Street South & North Street/165 Sandwich Street South Drille Was Sandwich Street South

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	₽		ሻ	₽	
Traffic Volume (vph)	4	0	5	4	0	4	4	708	0	3	684	7
Future Volume (vph)	4	0	5	4	0	4	4	708	0	3	684	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0		0.0	0.0		0.0	10.0		0.0	30.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.925			0.932						0.999	
Flt Protected		0.978			0.976		0.950			0.950		
Satd. Flow (prot)	0	1500	0	0	1676	0	1750	1842	0	1750	1840	0
Flt Permitted		0.978			0.976		0.950			0.950		
Satd. Flow (perm)	0	1500	0	0	1676	0	1750	1842	0	1750	1840	0
Link Speed (k/h)		50			30			50			50	
Link Distance (m)		127.1			58.9			86.3			124.2	
Travel Time (s)		9.2			7.1			6.2			8.9	
Confl. Peds. (#/hr)							8		1	1		8
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Parking (#/hr)		0										
Adj. Flow (vph)	4	0	5	4	0	4	4	722	0	3	698	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	9	0	0	8	0	4	722	0	3	705	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.01	1.16	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Stop			Stop			Free			Free	
Intono a 15 a 10												

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 47.3% ICU Level of Service A

Analysis Period (min) 15

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HCM 2010 TWSC 2022 PM Existing 3: Sandwich Street South & North Street/165 Sandwich Street South Drive & Sandwich Street & Sandwich

Intersection Int Delay, s/veh 0.4 **EBL WBR NBL NBT NBR** SBL Movement **EBT** EBR **WBL WBT SBT SBR** Lane Configurations 4 4 ₽ ኘ ₽ 0 Traffic Vol. veh/h 4 0 4 708 3 684 Future Vol, veh/h 4 0 5 4 0 4 4 708 0 3 684 7 0 0 0 Conflicting Peds, #/hr 0 0 0 8 0 8 0 Sign Control Stop Stop Stop Stop Stop Stop Free Free Free Free Free Free RT Channelized None None None None Storage Length 100 300 _ Veh in Median Storage, # 0 0 0 0 Grade, % 0 0 0 0 98 Peak Hour Factor 98 98 98 98 98 98 98 98 98 98 98 Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 Mvmt Flow 4 0 5 4 0 4 4 722 0 3 698 Major/Minor Minor2 Minor1 Major1 Major2 Conflicting Flow All 1448 1447 710 1441 1450 723 713 0 723 0 0 0 Stage 1 716 716 731 731 Stage 2 732 731 710 719 Critical Hdwy 7.12 6.52 6.22 7.12 6.52 6.22 4.12 4.12 Critical Hdwy Stg 1 6.12 6.12 5.52 5.52 Critical Hdwy Stg 2 6.12 5.52 6.12 5.52 Follow-up Hdwy 3.518 4.018 3.318 3.518 4.018 3.318 2.218 2.218 Pot Cap-1 Maneuver 109 131 434 110 131 426 887 879 Stage 1 421 434 413 427 ----_ Stage 2 413 427 424 433 Platoon blocked, % 129 Mov Cap-1 Maneuver 106 129 431 108 426 880 878 Mov Cap-2 Maneuver 106 129 108 129 _ _ _ 424 Stage 1 416 430 411 Stage 2 407 424 418 429 EB WB NB SB Approach HCM Control Delay, s 25.8 27 0.1 0 **HCM LOS** D D Minor Lane/Major Mvmt NBL NBT NBR EBLn1WBLn1 SBL SBT **SBR** Capacity (veh/h) 880 182 172 878 HCM Lane V/C Ratio 0.005 0.05 0.047 0.003 HCM Control Delay (s) 9.1 25.8 27 9.1 **HCM Lane LOS** D Α D Α

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0

0.2

0.1

0

HCM 95th %tile Q(veh)

Lanes, Volumes, Timings 4: Sandwich Street South & Pedestrian Crossing

	•	-	•	•	←	•	•	†	/	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations								^			†	
Traffic Volume (vph)	0	0	0	0	0	0	0	715	0	0	693	0
Future Volume (vph)	0	0	0	0	0	0	0	715	0	0	693	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt												
Flt Protected												
Satd. Flow (prot)	0	0	0	0	0	0	0	1842	0	0	1842	0
Flt Permitted	•	-	•	•	•		•		•			
Satd. Flow (perm)	0	0	0	0	0	0	0	1842	0	0	1842	0
Right Turn on Red		•	Yes			Yes			Yes	•		Yes
Satd. Flow (RTOR)												
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		31.3			34.9			124.2			35.2	
Travel Time (s)		2.3			2.5			8.9			2.5	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	730	0.00	0.00	707	0.00
Shared Lane Traffic (%)								700			101	
Lane Group Flow (vph)	0	0	0	0	0	0	0	730	0	0	707	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	Loit	0.0	rtigrit	Loit	0.0	rtigitt	LOIL	3.5	ragin	Loit	3.5	rtigrit
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane		0.0			0.0			Yes			Yes	
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	1.01	15	25	1.01	15	25	1.01	15	25	1.01	15
Number of Detectors	20		10	20		10	20	2	10	20	2	10
Detector Template								Thru			Thru	
Leading Detector (m)								10.0			10.0	
Trailing Detector (m)								0.0			0.0	
Detector 1 Position(m)								0.0			0.0	
Detector 1 Size(m)								0.6			0.6	
Detector 1 Type								CI+Fx			Cl+Fx	
Detector 1 Channel								OIILX			OIILX	
Detector 1 Extend (s)								0.0			0.0	
Detector 1 Queue (s)								0.0			0.0	
Detector 1 Delay (s)								0.0			0.0	
Detector 2 Position(m)								9.4			9.4	
Detector 2 Size(m)								0.6			0.6	
Detector 2 Type								Cl+Ex			CI+Ex	
Detector 2 Type Detector 2 Channel								OITEX			OIILX	
Detector 2 Extend (s)								0.0			0.0	
Turn Type								NA			NA	
Protected Phases								2			6	
Permitted Phases											U	
Detector Phase								2			6	
Switch Phase								۷			Ü	
Minimum Initial (s)								10.0			10.0	
iviii iiiiiiiiiiiiiiiiiiiiiiiiiiiiiiii								10.0			10.0	

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Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
FIt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Peak Hour Factor	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
• • •	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)								32.5			24.5	
Total Split (s)								35.0			35.0	
Total Split (%)								53.8%			53.8%	
Maximum Green (s)								28.5			28.5	
Yellow Time (s)								3.3			3.3	
All-Red Time (s)								3.2			3.2	
Lost Time Adjust (s)								0.0			0.0	
Total Lost Time (s)								6.5			6.5	
Lead/Lag								0.0			0.0	
Lead-Lag Optimize?												
Vehicle Extension (s)								3.0			3.0	
Recall Mode								Max			Max	
Walk Time (s)								7.0			7.0	
Flash Dont Walk (s)								11.0			11.0	
Pedestrian Calls (#/hr)								0			0	
								50.9			50.9	
Act Effct Green (s)												
Actuated g/C Ratio								0.91			0.91	
v/c Ratio								0.44			0.42	
Control Delay								5.5			5.3	
Queue Delay								0.0			0.0	
Total Delay								5.5			5.3	
LOS								A			A	
Approach Delay								5.5			5.3	
Approach LOS								Α			Α	
Queue Length 50th (m)								0.0			0.0	
Queue Length 95th (m)								119.8			113.5	
Internal Link Dist (m)		7.3			10.9			100.2			11.2	
Turn Bay Length (m)												
Base Capacity (vph)								1674			1674	
Starvation Cap Reductn								0			0	
Spillback Cap Reductn								0			0	
Storage Cap Reductn								0			0	
Reduced v/c Ratio								0.44			0.42	
Intersection Summary												
<i>y</i> 1	ther											
Cycle Length: 65												
Actuated Cycle Length: 56												
Natural Cycle: 65												
Control Type: Semi Act-Unco	ord											
Maximum v/c Ratio: 0.44												
Intersection Signal Delay: 5.4				In	itersection	n LOS: A						
Intersection Capacity Utilization	on 43.0%			IC	CU Level of	of Service	Α					
Analysis Period (min) 15												
Splits and Phases: 4: Sand	wich Stre	et South	& Pedest	rian Cros	sing	_	_			_	_	
↑ ø2						₹¥ø4						
35 s						30 s						
↓ Ø6												}
35 s												

Lane Group	Ø4
Minimum Split (s)	30.0
Total Split (s)	30.0
Total Split (%)	46%
Maximum Green (s)	26.0
Yellow Time (s)	3.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	19.0
Pedestrian Calls (#/hr)	4
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

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Intersection: 1: Sandwich Street South & Fort Street

Movement	WB	WB	NB	SB	SB
Directions Served	L	R	TR	L	Т
Maximum Queue (m)	16.6	11.5	2.7	12.8	39.2
Average Queue (m)	3.1	4.1	0.1	2.8	2.4
95th Queue (m)	10.8	11.7	1.7	10.1	17.5
Link Distance (m)		228.3	30.4		61.8
Upstream Blk Time (%)					0
Queuing Penalty (veh)					0
Storage Bay Dist (m)	20.0			30.0	
Storage Blk Time (%)	1	0			0
Queuing Penalty (veh)	0	0			0

Intersection: 2: Sandwich Street South & Parking Lot Driveway

Movement	WB	NB	SB	SB
Directions Served	LR	TR	L	T
Maximum Queue (m)	12.7	1.8	17.4	54.6
Average Queue (m)	4.9	0.1	1.3	14.2
95th Queue (m)	12.4	1.3	8.3	42.4
Link Distance (m)	124.3	25.9		30.4
Upstream Blk Time (%)			0	4
Queuing Penalty (veh)			0	19
Storage Bay Dist (m)			15.0	
Storage Blk Time (%)			0	7
Queuing Penalty (veh)			0	1

Intersection: 3: Sandwich Street South & North Street/165 Sandwich Street South Driveway

Movement	EB	WB	NB	NB	SB	SB
Directions Served	LTR	LTR	L	TR	L	TR
Maximum Queue (m)	12.8	7.3	18.8	10.2	8.9	2.9
Average Queue (m)	4.3	0.5	6.2	0.3	0.6	0.1
95th Queue (m)	12.0	3.8	15.9	4.7	4.4	1.5
Link Distance (m)	118.7	50.2		79.6		111.9
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (m)			10.0		30.0	
Storage Blk Time (%)			6	0		
Queuing Penalty (veh)			24	0		

Intersection: 4: Sandwich Street South & Pedestrian Crossing

Movement	NB	SB
Directions Served	T	T
Maximum Queue (m)	80.1	50.0
Average Queue (m)	40.5	38.7
95th Queue (m)	67.0	54.3
Link Distance (m)	111.9	25.9
Upstream Blk Time (%)		22
Queuing Penalty (veh)		117
Storage Bay Dist (m)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 162

Intersection: 1: Sandwich Street South & Fort Street

Movement	WB	WB	NB	SB	SB
Directions Served	L	R	TR	L	T
Maximum Queue (m)	18.7	14.3	7.9	10.3	3.3
Average Queue (m)	7.3	5.4	0.4	2.6	0.1
95th Queue (m)	15.9	13.8	3.9	9.4	1.7
Link Distance (m)		228.3	30.4		61.8
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (m)	20.0			30.0	
Storage Blk Time (%)	1	0			
Queuing Penalty (veh)	0	0			

Intersection: 2: Sandwich Street South & Parking Lot Driveway

Movement	WB	NB	SB
Directions Served	LR	TR	T
Maximum Queue (m)	10.0	10.3	19.8
Average Queue (m)	1.7	0.5	2.3
95th Queue (m)	7.3	5.1	13.9
Link Distance (m)	124.3	24.5	30.4
Upstream Blk Time (%)		0	0
Queuing Penalty (veh)		0	1
Storage Bay Dist (m)			
Storage Blk Time (%)			1
Queuing Penalty (veh)			0

Intersection: 3: Sandwich Street South & North Street/165 Sandwich Street South Driveway

Movement	EB	WB	NB	SB	
Directions Served	LTR	LTR	L	L	
Maximum Queue (m)	9.2	13.1	9.0	3.5	
Average Queue (m)	2.8	2.7	0.7	0.2	
95th Queue (m)	9.5	10.2	4.5	2.5	
Link Distance (m)	118.7	50.2			
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (m)			10.0	30.0	
Storage Blk Time (%)			0		
Queuing Penalty (veh)			2		

Intersection: 4: Sandwich Street South & Pedestrian Crossing

Movement	NB	SB
Directions Served	Ţ	T
Maximum Queue (m)	66.5	39.4
Average Queue (m)	5.2	3.6
95th Queue (m)	31.8	22.1
Link Distance (m)	114.0	24.5
Upstream Blk Time (%)		1
Queuing Penalty (veh)		8
Storage Bay Dist (m)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 12

Appendix F

Signal Warrants

Sandwich St S & Fort St 2023 FB

Justification #7

		Minimum R	equirement	Minimum R	equirement		Compliance		
Justification	Description	1 Lane I	Highway	2 or Mo	re Lanes	Secti	ional	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	LIILII 6 /0	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	627	87%	22%	No
Volume	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	38	22%	2270	NO
	A. Vehicle volumes, major street (average hour)	480	720	600	900	602	84%		
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	14	19%	19%	No

- 1. Refer to OTM Book 12, pg 92, Mar 2012
- 2. Lowest section percentage governs justification
- 3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors
- 4. T-intersection factor corrected, applies only to 1B

Sandwich St S & Parking Lot Driveway 2023 FB

Justification #7

		Minimum R	equirement	Minimum R	equirement		Compliance		
Justification	Description	1 Lane I	Highway	2 or Mo	re Lanes	Sect	ional	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	Littile /0	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	617	86%	7%	No
Volume	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	12	7%	7 70	NO
	A. Vehicle volumes, major street (average hour)	480	720	600	900	609	85%		
2. Delay to Cross Traffic	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	2	2%	2%	No

- 1. Refer to OTM Book 12, pg 92, Mar 2012
- 2. Lowest section percentage governs justification
- 3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors
- 4. T-intersection factor corrected, applies only to 1B

Sandwich St S & North St 2023 FB

Justification #7

		Minimum R	equirement	Minimum R	equirement		Compliance		
Justification	Description	1 Lane l	Highway	2 or Mo	re Lanes	Secti	ional	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	LIILII 6 /0	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	633	88%	6%	No
Volume	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	10	6%	0%	NO
	A. Vehicle volumes, major street (average hour)	480	720	600	900	623	86%		
2. Delay to Cross Traffic	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	3	4%	4%	No

- 1. Refer to OTM Book 12, pg 92, Mar 2012
- 2. Lowest section percentage governs justification
- 3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors
- 4. T-intersection factor corrected, applies only to 1B

Sandwich St S & Fort St 2023 FT

Justification #7

		Minimum R	equirement	Minimum R	Requirement		Compliance		
Justification	Description	1 Lane l	Highway	2 or Mo	re Lanes	Secti	ional	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	LIILII 6 /0	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	635	88%	23%	No
Volume	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	39	23%	25%	NO
	A. Vehicle volumes, major street (average hour)	480	720	600	900	609	85%		
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	15	20%	20%	No

- 1. Refer to OTM Book 12, pg 92, Mar 2012
- 2. Lowest section percentage governs justification
- 3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors
- 4. T-intersection factor corrected, applies only to 1B

Sandwich St S & Parking Lot Driveway 2023 FT

Justification #7

		Minimum R	equirement	Minimum R	equirement		Compliance		
Justification	Description	1 Lane I	Highway	2 or Mo	re Lanes	Sect	ional	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	Littile /0	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	629	87%	11%	No
Volume	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	18	11%	11%	NO
	A. Vehicle volumes, major street (average hour)	480	720	600	900	617	86%		
2. Delay to Cross Traffic	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	4	5%	5%	No

- 1. Refer to OTM Book 12, pg 92, Mar 2012
- 2. Lowest section percentage governs justification
- 3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors
- 4. T-intersection factor corrected, applies only to 1B

Sandwich St S & North St 2023 FT

Justification #7

		Minimum R	equirement	Minimum R	equirement		Compliance		
Justification	Description	1 Lane I	Highway	2 or Mo	re Lanes	Sect	ional	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	Littile /0	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	643	89%	6%	No
Volume	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	11	6%	0%	NO
	A. Vehicle volumes, major street (average hour)	480	720	600	900	632	88%		
2. Delay to Cross Traffic	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	4	5%	5%	No

- 1. Refer to OTM Book 12, pg 92, Mar 2012
- 2. Lowest section percentage governs justification
- 3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors
- 4. T-intersection factor corrected, applies only to 1B

Sandwich St S & Access #1 2023 FT

Justification #7

		Minimum R	equirement	Minimum R	Requirement		Compliance		
Justification	Description	1 Lane l	Highway	2 or Mo	re Lanes	Secti	ional	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	LIILII 6 /0	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	617	86%	3%	No
Volume	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	6	3%	370	NO
	A. Vehicle volumes, major street (average hour)	480	720	600	900	613	85%		
2. Delay to Cross Traffic	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	2	3%	3%	No

- 1. Refer to OTM Book 12, pg 92, Mar 2012
- 2. Lowest section percentage governs justification
- 3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors
- 4. T-intersection factor corrected, applies only to 1B

Sandwich St S & Access #2 2023 FT

Justification #7

		Minimum R	equirement	Minimum R	equirement		Compliance		
Justification	Description	1 Lane l	Highway	2 or Mo	re Lanes	Secti	onal	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	LIILII 6 /0	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	621	86%	3%	No
Volume	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	5	3%	5%	NO
	A. Vehicle volumes, major street (average hour)	480	720	600	900	618	86%		
2. Delay to Cross Traffic	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	2	2%	2%	No

- 1. Refer to OTM Book 12, pg 92, Mar 2012
- 2. Lowest section percentage governs justification
- 3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors
- 4. T-intersection factor corrected, applies only to 1B

Appendix G

2023 Future Background Synchro & SimTraffic Worksheets

Lanes, Volumes, Timings 1: Sandwich Street South & Fort Street

	•	•	†	/	>	↓	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	7	^		ሻ	†	
Traffic Volume (vph)	14	18	394	54	23	533	
Future Volume (vph)	14	18	394	54	23	533	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (m)	20.0	0.0		0.0	30.0		
Storage Lanes	1	1		0	1		
Taper Length (m)	15.0				15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor							
Frt		0.850	0.984				
Flt Protected	0.950				0.950		
Satd. Flow (prot)	1653	1566	1797	0	1750	1807	
Flt Permitted	0.950				0.950		
Satd. Flow (perm)	1653	1566	1797	0	1750	1807	
Link Speed (k/h)	40		50			50	
Link Distance (m)	238.1		44.2			71.5	
Travel Time (s)	21.4		3.2			5.1	
Confl. Peds. (#/hr)		1		5	5		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Heavy Vehicles (%)	8%	2%	3%	2%	2%	4%	
Adj. Flow (vph)	15	19	415	57	24	561	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	15	19	472	0	24	561	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Left	
Median Width(m)	3.5		3.5			3.5	
Link Offset(m)	0.0		0.0			0.0	
Crosswalk Width(m)	3.0		3.0			3.0	
Two way Left Turn Lane			Yes			Yes	
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01	
Turning Speed (k/h)	25	15		15	25		
Sign Control	Stop		Free			Free	
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	ion 38.4%			IC	CU Level o	of Service A	4

Analysis Period (min) 15

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2023 FB AM

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	7	\$	11511	ሻ	<u> </u>
Traffic Vol, veh/h	14	18	394	54	23	533
Future Vol, veh/h	14	18	394	54	23	533
Conflicting Peds, #/hr	0	1	0	5	5	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-	None	-	None
Storage Length	200	0	_	-	300	-
Veh in Median Storage		-	0	_	-	0
Grade, %	0	<u>-</u>	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	8	2	3	2	2	4
Mymt Flow	15	19	415	57	24	561
INIVITIL FIOW	10	19	415	51	24	301
Major/Minor N	Minor1	N	//ajor1	1	Major2	
Conflicting Flow All	1058	450	0	0	477	0
Stage 1	449	-	-	-	-	-
Stage 2	609	-	-	-	-	-
Critical Hdwy	6.48	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.48	-	-	-	-	-
Critical Hdwy Stg 2	5.48	-	-	-	-	-
Follow-up Hdwy	3.572	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	242	609	-	-	1085	-
Stage 1	631	-	-	-	-	-
Stage 2	531	-	-	-	-	-
Platoon blocked, %			-	_		_
Mov Cap-1 Maneuver	235	606	-	-	1080	-
Mov Cap-2 Maneuver	365	-	_	_	-	_
Stage 1	628	_	_	_	_	_
Stage 2	519	_	_	_	_	_
Olage 2	010					
Approach	WB		NB		SB	
HCM Control Delay, s	12.9		0		0.3	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1V	VBI n2	SBL
Capacity (veh/h)	<u></u>		-	365	606	1080
HCM Lane V/C Ratio		<u>-</u>	_		0.031	
HCM Control Delay (s)		_	_	15.3	11.1	8.4
HCM Lane LOS		_	_	13.3 C	В	Α
HCM 95th %tile Q(veh))	_	_	0.1	0.1	0.1
HOW JOHN JOHN Q(VEII)				U. I	U. I	0.1

		_		
2: Sandwich	Street	South	& Parking	Lot Driveway

	•	•	1	~	-	¥
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		f)		*	
Traffic Volume (vph)	2	22	425	33	15	531
Future Volume (vph)	2	22	425	33	15	531
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0		0.0	15.0	
Storage Lanes	1	0		0	1	
Taper Length (m)	15.0				15.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.876		0.990			
Flt Protected	0.996				0.950	
Satd. Flow (prot)	1607	0	1807	0	1750	1807
Flt Permitted	0.996			•	0.950	
Satd. Flow (perm)	1607	0	1807	0	1750	1807
Link Speed (k/h)	30	•	50	•		50
Link Distance (m)	133.6		37.5			44.2
Travel Time (s)	16.0		2.7			3.2
Confl. Peds. (#/hr)	10.0		,	19	19	0.2
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	2%	2%	3%	2%	2%	4%
Adj. Flow (vph)	2	23	443	34	16	553
Shared Lane Traffic (%)		20	110	01	10	000
Lane Group Flow (vph)	25	0	477	0	16	553
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5	ragnt	3.5	ragnt	LUIL	3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane	3.0		Yes			Yes
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	1.01	1.01	1.01	25	1.01
Sign Control	Stop	10	Free	10	20	Free
	διυρ		FIEE			FIEE
Intersection Summary						
Area Type: (Other					
Control Type: Unsignalized						

Control Type: Unsignalized Intersection Capacity Utilization 37.9%

ICU Level of Service A

Analysis Period (min) 15

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131 & 135 Sandwich Street South

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	ĵ.		7	f)	
Traffic Volume (vph)	3	0	18	2	0	0	62	449	5	6	507	28
Future Volume (vph)	3	0	18	2	0	0	62	449	5	6	507	28
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0		0.0	0.0		0.0	10.0		0.0	30.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.883						0.998			0.992	
Flt Protected		0.994			0.950		0.950			0.950		
Satd. Flow (prot)	0	1455	0	0	1750	0	1653	1821	0	1750	1776	0
Flt Permitted		0.994			0.950		0.950			0.950		
Satd. Flow (perm)	0	1455	0	0	1750	0	1653	1821	0	1750	1776	0
Link Speed (k/h)		50			30			50			50	
Link Distance (m)		127.1			58.9			86.3			121.9	
Travel Time (s)		9.2			7.1			6.2			8.8	
Confl. Peds. (#/hr)			1	1			9		26	26		9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	8%	3%	2%	2%	5%	4%
Parking (#/hr)		0										
Adj. Flow (vph)	3	0	20	2	0	0	69	499	6	7	563	31
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	23	0	0	2	0	69	505	0	7	594	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.01	1.16	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
	ther											
Control Type: Unsignalized												
Intersection Capacity Utilization	on 45.5%	1		IC	CU Level	of Service	Α					
Analysis Period (min) 15												

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HCM 2010 TWSC 2023 FB AM

131 & 135 Sandwich Street South

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	f)		ሻ	î,	
Traffic Vol, veh/h	3	0	18	2	0	0	62	449	5	6	507	28
Future Vol, veh/h	3	0	18	2	0	0	62	449	5	6	507	28
Conflicting Peds, #/hr	0	0	1	1	0	0	9	0	26	26	0	9
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	_	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	300	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	8	3	2	2	5	4
Mvmt Flow	3	0	20	2	0	0	69	499	6	7	563	31
Major/Minor I	Minor2			Minor1			Major1		<u> </u>	Major2		
Conflicting Flow All	1242	1271	589	1270	1283	528	603	0	0	531	0	0
Stage 1	602	602	-	666	666	-	-	-	-	-	-	-
Stage 2	640	669	-	604	617	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.18	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.272	-	-	2.218	-	-
Pot Cap-1 Maneuver	152	168	508	145	165	550	946	-	-	1036	-	-
Stage 1	486	489	-	449	457	-	-	-	-	-	-	-
Stage 2	464	456	-	485	481	-	_	-	-	-	-	_
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	142	150	503	127	147	537	938	-	-	1011	-	-
Mov Cap-2 Maneuver	142	150	-	127	147	-	-	-	-	-	-	-
Stage 1	447	482	-	406	413	-	-	-	-	-	-	-
Stage 2	430	412	-	462	474	-	-	-	-	-	-	-
, in the second second												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	15.4			33.9			1.1			0.1		
HCM LOS	С			D								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		938	-	-	369	127	1011	-	-			
HCM Lane V/C Ratio		0.073	_	-	0.063			_	_			
HCM Control Delay (s)		9.1	-	-	15.4	33.9	8.6	-	_			
HCM Lane LOS		A	_	_	С	D	A	_	_			
HCM 95th %tile Q(veh)	0.2	-	-	0.2	0.1	0	-	_			
3,000												

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations								*			*	
Traffic Volume (vph)	0	0	0	0	0	0	0	458	0	0	533	0
Future Volume (vph)	0	0	0	0	0	0	0	458	0	0	533	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt												
Flt Protected												
Satd. Flow (prot)	0	0	0	0	0	0	0	1842	0	0	1842	0
Flt Permitted	•	•				•						•
Satd. Flow (perm)	0	0	0	0	0	0	0	1842	0	0	1842	0
Right Turn on Red		· ·	Yes	•	•	Yes	•	1012	Yes	•	1012	Yes
Satd. Flow (RTOR)			. 00			. 00			. 00			. 00
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		27.0			26.6			121.9			37.5	
Travel Time (s)		1.9			1.9			8.8			2.7	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	0.50	0.30	0.50	0.50	0.50	0.50	0.50	477	0.50	0.50	555	0.50
Shared Lane Traffic (%)	U	U	U	U	U	U	U	7//	U	U	333	U
Lane Group Flow (vph)	0	0	0	0	0	0	0	477	0	0	555	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	LOIL	0.0	rtigrit	Leit	0.0	rtigrit	Leit	3.5	ragnt	Leit	3.5	rtigrit
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane		3.0			3.0			Yes			Yes	
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	1.01	1.01	25	1.01	1.01	25	1.01	1.01	25	1.01	1.01
Number of Detectors	20		10	20		15	20	2	10	23	2	10
Detector Template								Thru			Thru	
								10.0			10.0	
Leading Detector (m)								0.0			0.0	
Trailing Detector (m) Detector 1 Position(m)								0.0			0.0	
` ,								0.0			0.0	
Detector 1 Size(m)												
Detector 1 Type								CI+Ex			Cl+Ex	
Detector 1 Channel								0.0			0.0	
Detector 1 Extend (s)								0.0			0.0	
Detector 1 Queue (s)								0.0			0.0	
Detector 1 Delay (s)								0.0			0.0	
Detector 2 Position(m)								9.4			9.4	
Detector 2 Size(m)								0.6			0.6	
Detector 2 Type								Cl+Ex			Cl+Ex	
Detector 2 Channel								0.0			0.0	
Detector 2 Extend (s)								0.0			0.0	
Turn Type								NA			NA	
Protected Phases								2			6	
Permitted Phases												
Detector Phase								2			6	
Switch Phase								16.5			4	
Minimum Initial (s)								10.0			10.0	

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Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Peak Hour Factor	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	40.0
Minimum Initial (s)	10.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)								24.5			24.5	
Total Split (s)								30.0			30.0	
Total Split (%)								50.0%			50.0%	
Maximum Green (s)								23.5			23.5	
Yellow Time (s)								3.3			3.3	
All-Red Time (s)								3.2			3.2	
Lost Time Adjust (s)								0.0			0.0	
Total Lost Time (s)								6.5			6.5	
Lead/Lag								0.0			0.0	
Lead-Lag Optimize?												
Vehicle Extension (s)								3.0			3.0	
Recall Mode								Max			Max	
Walk Time (s)								7.0			7.0	
Flash Dont Walk (s)								11.0			11.0	
Pedestrian Calls (#/hr)								0			0	
								42.1			42.1	
Act Effct Green (s)								0.61			0.61	
Actuated g/C Ratio												
v/c Ratio								0.42			0.49	
Control Delay								12.6			13.7	
Queue Delay								0.0			0.0	
Total Delay								12.6			13.7	
LOS								В			В	
Approach Delay								12.6			13.7	
Approach LOS								В			В	
Queue Length 50th (m)								41.5			51.3	
Queue Length 95th (m)								65.1			79.9	
Internal Link Dist (m)		3.0			2.6			97.9			13.5	
Turn Bay Length (m)												
Base Capacity (vph)								1124			1124	
Starvation Cap Reductn								0			0	
Spillback Cap Reductn								0			0	
Storage Cap Reductn								0			0	
Reduced v/c Ratio								0.42			0.49	
Intersection Summary												
	Other											
Cycle Length: 60	Otrici											
Actuated Cycle Length: 69												
Natural Cycle: 60												
Control Type: Semi Act-Und	oord											
Maximum v/c Ratio: 0.49	Jooru											
Intersection Signal Delay: 13	2 2			ln	tersection	N OC D						
						of Service	. ^					
Intersection Capacity Utiliza Analysis Period (min) 15	IIIUII 33.5%			IC	O LEVEI (JI JEI VICE						
Analysis Pellou (IIIIII) 15												
Splits and Phases: 4: Sar	ndwich Stre	et South	& Pedest	rian Cros	sing							
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Lane Group	Ø4
Minimum Split (s)	30.0
Total Split (s)	30.0
Total Split (%)	50%
Maximum Green (s)	26.0
Yellow Time (s)	3.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	19.0
Pedestrian Calls (#/hr)	137
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Lanes, Volumes, Timings 1: Sandwich Street South & Fort Street

	•	•	†	/	>	ļ	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	7	^		ሻ	†	
Traffic Volume (vph)	43	26	646	81	14	661	
Future Volume (vph)	43	26	646	81	14	661	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (m)	20.0	0.0		0.0	30.0		
Storage Lanes	1	1		0	1		
Taper Length (m)	15.0				15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor							
Frt		0.850	0.985				
Flt Protected	0.950				0.950		
Satd. Flow (prot)	1750	1566	1814	0	1750	1842	
Flt Permitted	0.950				0.950		
Satd. Flow (perm)	1750	1566	1814	0	1750	1842	
Link Speed (k/h)	40		50			50	
Link Distance (m)	238.1		44.2			71.5	
Travel Time (s)	21.4		3.2			5.1	
Confl. Peds. (#/hr)	1			5	5		
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	
Adj. Flow (vph)	44	27	666	84	14	681	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	44	27	750	0	14	681	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Left	
Median Width(m)	3.5		3.5			3.5	
Link Offset(m)	0.0		0.0			0.0	
Crosswalk Width(m)	3.0		3.0			3.0	
Two way Left Turn Lane			Yes			Yes	
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01	
Turning Speed (k/h)	25	15		15	25		
Sign Control	Stop		Free			Free	
Intersection Summary							
	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	ion 49.0%			IC	CU Level o	of Service A	Α
Analysis Period (min) 15							

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2023 FB PM

Intersection						
Int Delay, s/veh	0.9					
		WED	NET	NDD	ODL	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	ች	7	\$	0.1	ች	↑
Traffic Vol, veh/h	43	26	646	81	14	661
Future Vol, veh/h	43	26	646	81	14	661
Conflicting Peds, #/hr	1	0	0	5	5	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	200	0	-	-	300	-
Veh in Median Storage		-	0	_	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	44	27	666	84	14	681
Major/Minor	Minor1		/lajor1	N	Major2	
Conflicting Flow All	1423	713	0	0	755	0
Stage 1	713	-	-	-	-	-
Stage 2	710	-	-	-	- 4.40	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy		3.318	-	-	2.218	-
Pot Cap-1 Maneuver	150	432	-	-	855	-
Stage 1	486	-	-	-	-	-
Stage 2	487	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	147	430	-	_	851	-
Mov Cap-2 Maneuver	287	-	-	-	-	-
Stage 1	484	-	-	-	-	-
Stage 2	479	-	-	-	-	-
Annroach	WB		NB		SB	
Approach						
HCM Control Delay, s	17.6		0		0.2	
HCM LOS	С					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1V	VBLn2	SBL
Capacity (veh/h)		_	-	287	430	851
HCM Lane V/C Ratio		-	_	0.154		
HCM Control Delay (s)		_	_	19.8	13.9	9.3
HCM Lane LOS		_	_	C	В	A
HCM 95th %tile Q(veh)	_	-	0.5	0.2	0.1
	7			3.0	J.L	J. 1

Lanes, Volumes, Timings 2: Sandwich Street South & Parking Lot Driveway

	•	•	†	/	>	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		ĵ»		ሻ	†
Traffic Volume (vph)	4	3	723	6	0	703
Future Volume (vph)	4	3	723	6	0	703
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0		0.0	15.0	
Storage Lanes	1	0		0	1	
Taper Length (m)	15.0				15.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.942		0.999			
Flt Protected	0.972					
Satd. Flow (prot)	1687	0	1840	0	1842	1842
Flt Permitted	0.972					
Satd. Flow (perm)	1687	0	1840	0	1842	1842
Link Speed (k/h)	30		50			50
Link Distance (m)	133.6		35.2			44.2
Travel Time (s)	16.0		2.5			3.2
Confl. Peds. (#/hr)	4	1		4	4	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	4	3	738	6	0	717
Shared Lane Traffic (%)						
Lane Group Flow (vph)	7	0	744	0	0	717
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		3.5			3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	15		15	25	
Sign Control	Stop		Free			Free
Intersection Summary						
	Other					
Control Type: Unsignalized	JUI GI					
7.	ion 19 70/			10	Hlevela	of Service A
Intersection Capacity Utilizat	1011 40.1 %			IU	U Level C	i service A
Analysis Period (min) 15						

03-11-2022 CGH Transportation Page 3 RM

2023 FB PM

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1		ሻ	<u>□ □ □ □</u>
Traffic Vol, veh/h	4	3	723	6	0	703
Future Vol, veh/h	4	3	723	6	0	703
Conflicting Peds, #/hr	4	1	0	4	4	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None	-		-	None
Storage Length	0	-	_	-	150	-
Veh in Median Storage		_	0	_	-	0
Grade, %	0	<u>-</u>	0	_	<u>-</u>	0
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	3	738	6	0	717
IVIVIIIL FIOW	4	ა	130	Ö	U	7.17
Major/Minor I	Minor1	N	Major1	ı	Major2	
Conflicting Flow All	1466	746	0	0	748	0
Stage 1	745	-	_	-	-	-
Stage 2	721	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	_	_	-
Critical Hdwy Stg 2	5.42	_	-	-	-	_
Follow-up Hdwy		3.318	-	_	2.218	_
Pot Cap-1 Maneuver	141	413	-	-	861	_
Stage 1	469	-	_	_	-	_
Stage 2	482	_	_	_	_	_
Platoon blocked, %	102		_	_		_
Mov Cap-1 Maneuver	140	411	_	_	858	_
Mov Cap-2 Maneuver	280	-	_	_	-	_
Stage 1	467	_			_	
Stage 2	480	_	_	_	_	_
Stage 2	400	-	-	_	_	_
Approach	WB		NB		SB	
HCM Control Delay, s	16.4		0		0	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NBT	NRRV	VBLn1	SBL	SBT
Capacity (veh/h)		1101		324	858	- UD
HCM Lane V/C Ratio		<u>-</u>		0.022	- 000	-
HCM Control Delay (s)		_	-	16.4	0	_
HCM Lane LOS		_	_	C	A	_
HCM 95th %tile Q(veh)	١		_	0.1	0	
				U. I	U	

131 & 135 Sandwich Street South

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		*	ĵ»		, j	f.	
Traffic Volume (vph)	4	0	5	4	0	4	4	722	0	3	698	7
Future Volume (vph)	4	0	5	4	0	4	4	722	0	3	698	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0		0.0	0.0		0.0	10.0		0.0	30.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.925			0.932						0.999	
Flt Protected		0.978			0.976		0.950			0.950		
Satd. Flow (prot)	0	1500	0	0	1676	0	1750	1842	0	1750	1840	0
Flt Permitted		0.978			0.976		0.950			0.950		
Satd. Flow (perm)	0	1500	0	0	1676	0	1750	1842	0	1750	1840	0
Link Speed (k/h)		50			30			50			50	
Link Distance (m)		127.1			58.9			86.3			124.2	
Travel Time (s)		9.2			7.1			6.2			8.9	
Confl. Peds. (#/hr)							8		1	1		8
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Parking (#/hr)		0										
Adj. Flow (vph)	4	0	5	4	0	4	4	737	0	3	712	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	9	0	0	8	0	4	737	0	3	719	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.01	1.16	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 48.0%

ICU Level of Service A

Analysis Period (min) 15

HCM 2010 TWSC 2023 FB PM

131 & 135 Sandwich Street South

Int Delay, s/veh	Intersection												
Traffic Vol, veh/h	Int Delay, s/veh	0.4											
Traffic Vol, veh/h	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	Lane Configurations		4			4		*	ĵ.		7	î,	
Conflicting Peds, #/hr	Traffic Vol, veh/h	4		5	4		4	4		0			7
Stop Control Stop Free Free	Future Vol, veh/h	4	0	5	4	0	4	4	722	0	3	698	7
RT Channelized	Conflicting Peds, #/hr	0	0	0	0	0	0	8	0	1	1	0	8
Storage Length	Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Veh in Median Storage, # - 0	RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Grade, %	Storage Length	-	-	-	-	-	-	100	-	-	300	-	-
Peak Hour Factor 98 98 98 98 98 98 98 9	Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2	Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Momit Flow 4 0 5 4 0 4 4 737 0 3 712 7 Major/Minor Minor1 Major1 Major2 Conflicting Flow All 1477 1476 724 1470 1479 738 727 0 0 738 0 0 Stage 1 730 730 - 746 746 -	Peak Hour Factor	98	98	98	98	98	98	98	98	98	98	98	98
Major/Minor Minor2 Minor1 Major1 Major2	Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Conflicting Flow All	Mvmt Flow	4	0	5	4	0	4	4	737	0	3	712	7
Conflicting Flow All													
Conflicting Flow All	Maior/Minor	Minor2			Minor1			Maior1		N	Maior2		
Stage 1			1476			1479			0			0	0
Stage 2									-	-	-		-
Critical Hdwy 7.12 6.52 6.22 7.12 6.52 6.22 4.12 - 4.12 - - Critical Hdwy Stg 1 6.12 5.52 - 6.12 5.52 -							_	_	_	_	_	_	_
Critical Hdwy Stg 1 6.12 5.52 - 6.12 5.52 -	•			6.22			6.22	4.12	-	_	4.12	-	_
Critical Hdwy Stg 2 6.12 5.52 - 6.12 5.52							-	-	_	_	-	-	_
Follow-up Hdwy 3.518 4.018 3.318 3.518 4.018 3.318 2.218 2.218 2.218 Pot Cap-1 Maneuver 104 126 426 105 126 418 876 868 Stage 1 414 428 - 405 421 Stage 2 405 421 - 417 426	, ,			-			-	_	-	_	-	-	_
Pot Cap-1 Maneuver				3.318			3.318	2.218	_	_	2.218	-	_
Stage 1 414 428 - 405 421 - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - -									-	_		-	-
Stage 2 405 421 - 417 426 -	•						-	-	_	_	-	-	_
Platoon blocked, %				-			-	_	-	_	-	-	-
Mov Cap-1 Maneuver 102 124 423 103 124 418 869 - - 867 - - Mov Cap-2 Maneuver 102 124 - 103 124 -	<u> </u>								_	_		_	_
Mov Cap-2 Maneuver 102 124 - 103 124 - </td <td></td> <td>102</td> <td>124</td> <td>423</td> <td>103</td> <td>124</td> <td>418</td> <td>869</td> <td>_</td> <td>_</td> <td>867</td> <td>_</td> <td>_</td>		102	124	423	103	124	418	869	_	_	867	_	_
Stage 1 409 424 - 403 418	· · · · · · · · · · · · · · · · · · ·							-	_	_		_	_
Stage 2 399 418 - 411 422 -	•						-	-	-	-	-	_	_
Approach EB WB NB SB HCM Control Delay, s 26.6 28 0.1 0 HCM LOS D D D D Minor Lane/Major Mvmt NBL NBT NBR EBLn1WBLn1 SBL SBT SBR Capacity (veh/h) 869 - - 176 165 867 - - HCM Lane V/C Ratio 0.005 - - 0.052 0.049 0.004 - - HCM Control Delay (s) 9.2 - - 26.6 28 9.2 - - HCM Lane LOS A - D D A - -	•						_	_	_	_	_	_	_
HCM Control Delay, s 26.6 28 0.1 0	2.0.30 _	300											
HCM Control Delay, s 26.6 28 0.1 0	Δnnroach	FP			\MP			NR			SB		
Minor Lane/Major Mvmt NBL NBT NBR EBLn1WBLn1 SBL SBT SBR Capacity (veh/h) 869 - - 176 165 867 - - HCM Lane V/C Ratio 0.005 - - 0.049 0.004 - - HCM Control Delay (s) 9.2 - - 26.6 28 9.2 - - HCM Lane LOS A - - D D A - -													
Minor Lane/Major Mvmt NBL NBT NBR EBLn1WBLn1 SBL SBT SBR Capacity (veh/h) 869 - - 176 165 867 - - HCM Lane V/C Ratio 0.005 - - 0.052 0.049 0.004 - - HCM Control Delay (s) 9.2 - - 26.6 28 9.2 - - HCM Lane LOS A - D D A - -								U. I			U		
Capacity (veh/h) 869 176 165 867 HCM Lane V/C Ratio 0.005 0.052 0.049 0.004 HCM Control Delay (s) 9.2 26.6 28 9.2 HCM Lane LOS A - D D A	TIOWI LOG	U			D								
Capacity (veh/h) 869 176 165 867 HCM Lane V/C Ratio 0.005 0.052 0.049 0.004 HCM Control Delay (s) 9.2 26.6 28 9.2 HCM Lane LOS A - D D A	Minor Lang/Major Mum	1	NDI	NDT	NDD	EDI 54V	VDI 51	CDI	CDT	CDD			
HCM Lane V/C Ratio 0.005 - - 0.052 0.049 0.004 - - HCM Control Delay (s) 9.2 - - 26.6 28 9.2 - - HCM Lane LOS A - - D D A - -		IL								SDK			
HCM Control Delay (s) 9.2 - - 26.6 28 9.2 - - HCM Lane LOS A - - D D A - -	1 3 \ /								-	-			
HCM Lane LOS A D D A									-	-			
HUNI 95th %tile Q(ven) 0 0.2 0.2 0		\								-			
	HCM 95th %tile Q(veh)	U	-	-	0.2	0.2	U	-	-			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations								*			^	
Traffic Volume (vph)	0	0	0	0	0	0	0	729	0	0	707	0
Future Volume (vph)	0	0	0	0	0	0	0	729	0	0	707	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt												
Flt Protected												
Satd. Flow (prot)	0	0	0	0	0	0	0	1842	0	0	1842	0
Flt Permitted	•	•				•						•
Satd. Flow (perm)	0	0	0	0	0	0	0	1842	0	0	1842	0
Right Turn on Red		· ·	Yes	•	•	Yes	•	1012	Yes		1012	Yes
Satd. Flow (RTOR)			. 00			. 00			. 00			. 00
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		31.3			34.9			124.2			35.2	
Travel Time (s)		2.3			2.5			8.9			2.5	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	0.50	0.30	0.50	0.50	0.50	0.50	0.50	744	0.50	0.50	721	0.50
Shared Lane Traffic (%)	U	U	U	U	U	U	U	777	U	U	121	U
Lane Group Flow (vph)	0	0	0	0	0	0	0	744	0	0	721	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	LOIL	0.0	Nigrit	Leit	0.0	rtigrit	Leit	3.5	ragnt	Leit	3.5	rtigrit
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane		3.0			3.0			Yes			Yes	
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	1.01	1.01	25	1.01	1.01	25	1.01	1.01	25	1.01	1.01
Number of Detectors	20		10	20		15	20	2	10	23	2	10
								Thru			Thru	
Detector Template Leading Detector (m)								10.0			10.0	
· ,								0.0			0.0	
Trailing Detector (m) Detector 1 Position(m)								0.0			0.0	
` ,								0.0			0.0	
Detector 1 Size(m)								Cl+Ex				
Detector 1 Type								CI+EX			Cl+Ex	
Detector 1 Channel								0.0			0.0	
Detector 1 Extend (s)								0.0			0.0	
Detector 1 Queue (s)								0.0			0.0	
Detector 1 Delay (s)								0.0			0.0	
Detector 2 Position(m)								9.4			9.4	
Detector 2 Size(m)								0.6			0.6	
Detector 2 Type								Cl+Ex			Cl+Ex	
Detector 2 Channel								0.0			0.0	
Detector 2 Extend (s)								0.0			0.0	
Turn Type								NA			NA	
Protected Phases								2			6	
Permitted Phases												
Detector Phase								2			6	
Switch Phase								16.5			4	
Minimum Initial (s)								10.0			10.0	

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Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Peak Hour Factor	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	40.0
Minimum Initial (s)	10.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)								32.5			24.5	
Total Split (s)								35.0			35.0	
Total Split (%)								53.8%			53.8%	
Maximum Green (s)								28.5			28.5	
Yellow Time (s)								3.3			3.3	
All-Red Time (s)								3.2			3.2	
Lost Time Adjust (s)								0.0			0.0	
Total Lost Time (s)								6.5			6.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)								3.0			3.0	
Recall Mode								Max			Max	
Walk Time (s)								7.0			7.0	
Flash Dont Walk (s)								11.0			11.0	
Pedestrian Calls (#/hr)								0			0	
Act Effct Green (s)								50.9			50.9	
Actuated g/C Ratio								0.91			0.91	
v/c Ratio								0.44			0.43	
Control Delay								5.6			5.4	
								0.0			0.0	
Queue Delay								5.6			5.4	
Total Delay												
LOS								A 5.6			A 5.4	
Approach Delay												
Approach LOS								A 0.0			A	
Queue Length 50th (m)											0.0	
Queue Length 95th (m)		7.0			40.0			124.1			117.2	
Internal Link Dist (m)		7.3			10.9			100.2			11.2	
Turn Bay Length (m)								4074			4074	
Base Capacity (vph)								1674			1674	
Starvation Cap Reductn								0			0	
Spillback Cap Reductn								0			0	
Storage Cap Reductn								0			0	
Reduced v/c Ratio								0.44			0.43	
Intersection Summary												
Area Type:	Other											
Cycle Length: 65												
Actuated Cycle Length: 56												
Natural Cycle: 65												
Control Type: Semi Act-Un	coord											
Maximum v/c Ratio: 0.44												
Intersection Signal Delay:	5.5			In	tersection	n LOS: A						
Intersection Capacity Utiliz	ation 43.8%			IC	U Level	of Service	Α					
Analysis Period (min) 15												
Splits and Phases: 4: Sa	andwich Stree	et South	& Pedest	rian Cros	sina							
*			34001	0,00		1.1						
I Ø2						₹kø4						
35 s						30 s						
₩ Ø6					l							
35 s												

Lane Group	Ø4
Minimum Split (s)	30.0
Total Split (s)	30.0
Total Split (%)	46%
Maximum Green (s)	26.0
Yellow Time (s)	3.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	19.0
Pedestrian Calls (#/hr)	4
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Intersection: 1: Sandwich Street South & Fort Street

Movement	WB	WB	NB	SB	SB
Directions Served	L	R	TR	L	T
Maximum Queue (m)	14.8	10.0	3.2	11.5	16.5
Average Queue (m)	2.7	3.9	0.2	2.4	0.9
95th Queue (m)	9.5	11.2	1.9	9.2	8.9
Link Distance (m)		228.3	30.4		61.8
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (m)	20.0			30.0	
Storage Blk Time (%)	0				0
Queuing Penalty (veh)	0				0

Intersection: 2: Sandwich Street South & Parking Lot Driveway

Movement	WB	NB	SB	SB
Directions Served	LR	TR	L	T
Maximum Queue (m)	15.5	2.0	25.6	48.0
Average Queue (m)	5.3	0.1	2.9	12.3
95th Queue (m)	13.0	1.4	13.0	38.1
Link Distance (m)	124.3	25.9		30.4
Upstream Blk Time (%)			0	3
Queuing Penalty (veh)			0	14
Storage Bay Dist (m)			15.0	
Storage Blk Time (%)			0	5
Queuing Penalty (veh)			0	1

Intersection: 3: Sandwich Street South & North Street/165 Sandwich Street South

Movement	EB	WB	NB	NB	SB	SB
Directions Served	LTR	LTR	L	TR	L	TR
Maximum Queue (m)	11.7	7.2	19.0	16.3	5.4	7.2
Average Queue (m)	4.6	0.6	6.8	0.7	0.6	0.2
95th Queue (m)	12.0	4.3	16.7	9.1	4.1	3.4
Link Distance (m)	118.7	50.2		79.6		111.9
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (m)			10.0		30.0	
Storage Blk Time (%)			7	0		
Queuing Penalty (veh)			31	0		

Intersection: 4: Sandwich Street South & Pedestrian Crossing

Movement	NB	SB
Directions Served	Т	T
Maximum Queue (m)	80.8	49.8
Average Queue (m)	39.3	38.0
95th Queue (m)	68.2	55.5
Link Distance (m)	111.9	25.9
Upstream Blk Time (%)		21
Queuing Penalty (veh)		112
Storage Bay Dist (m)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 157

Intersection: 1: Sandwich Street South & Fort Street

Movement	WB	WB	NB	SB	SB
Directions Served	L	R	TR	L	Т
Maximum Queue (m)	21.6	20.4	5.0	9.2	13.1
Average Queue (m)	8.6	6.6	0.2	2.4	0.6
95th Queue (m)	17.6	15.4	2.1	9.1	7.4
Link Distance (m)		228.3	30.4		61.8
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (m)	20.0			30.0	
Storage Blk Time (%)	2	0			0
Queuing Penalty (veh)	0	0			0

Intersection: 2: Sandwich Street South & Parking Lot Driveway

Movement	WB	NB	SB
Directions Served	LR	TR	T
Maximum Queue (m)	10.1	8.0	28.2
Average Queue (m)	1.7	0.5	2.4
95th Queue (m)	7.4	5.5	14.4
Link Distance (m)	124.3	24.5	30.4
Upstream Blk Time (%)		0	0
Queuing Penalty (veh)		0	1
Storage Bay Dist (m)			
Storage Blk Time (%)			1
Queuing Penalty (veh)			0

Intersection: 3: Sandwich Street South & North Street/165 Sandwich Street South

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	L	L
Maximum Queue (m)	10.5	10.4	7.2	5.3
Average Queue (m)	2.5	3.2	0.7	0.2
95th Queue (m)	9.2	10.3	4.7	2.6
Link Distance (m)	118.7	50.2		
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (m)			10.0	30.0
Storage Blk Time (%)			0	
Queuing Penalty (veh)			2	

Intersection: 4: Sandwich Street South & Pedestrian Crossing

Movement	NB	SB
Directions Served	Т	Т
Maximum Queue (m)	68.5	44.0
Average Queue (m)	8.0	6.0
95th Queue (m)	40.5	29.5
Link Distance (m)	114.0	24.5
Upstream Blk Time (%)		2
Queuing Penalty (veh)		15
Storage Bay Dist (m)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 19

Appendix H

2023 Future Total Synchro & SimTraffic Worksheets

Lanes, Volumes, Timings 1: Sandwich Street South & Fort Street

	•	4	†	/	>	ļ	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	7	ĵ.		ሻ	†	
Traffic Volume (vph)	15	18	395	55	23	534	
Future Volume (vph)	15	18	395	55	23	534	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (m)	20.0	0.0		0.0	30.0		
Storage Lanes	1	1		0	1		
Taper Length (m)	15.0				15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor							
Frt		0.850	0.983				
Flt Protected	0.950				0.950		
Satd. Flow (prot)	1653	1566	1795	0	1750	1807	
Flt Permitted	0.950				0.950		
Satd. Flow (perm)	1653	1566	1795	0	1750	1807	
Link Speed (k/h)	40		50			50	
Link Distance (m)	238.1		44.2			71.5	
Travel Time (s)	21.4		3.2			5.1	
Confl. Peds. (#/hr)		1		5	5		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Heavy Vehicles (%)	8%	2%	3%	2%	2%	4%	
Adj. Flow (vph)	16	19	416	58	24	562	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	16	19	474	0	24	562	
Enter Blocked Intersection	No	No	Yes	No	No	Yes	
Lane Alignment	Left	Right	Left	Right	Left	Left	
Median Width(m)	3.5		3.5			3.5	
Link Offset(m)	0.0		0.0			0.0	
Crosswalk Width(m)	3.0		3.0			3.0	
Two way Left Turn Lane			Yes			Yes	
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01	
Turning Speed (k/h)	25	15		15	25		
Sign Control	Stop		Free			Free	
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat		ICU Level of Service A					
1 1 5 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1							

Analysis Period (min) 15

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Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	VVDL	VVDIX	1\D1	NOI	JDL Š	<u>361</u>
Traffic Vol, veh/h	15	18	395	55	23	534
Future Vol, veh/h	15	18	395	55	23	534
Conflicting Peds, #/hr	0	10	0	5	5	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-		-	None
Storage Length	200	0	_	-	300	-
Veh in Median Storage		-	0	_	-	0
Grade, %	, # 0 0	<u>-</u>	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
	8	2	3	2	2	4
Heavy Vehicles, % Mvmt Flow	16	19	416	58	24	562
INIVITIL FIOW	10	19	410	50	24	502
Major/Minor N	Minor1	N	Major1	1	Major2	
Conflicting Flow All	1060	451	0	0	479	0
Stage 1	450	-	-	-	-	-
Stage 2	610	-	-	-	-	-
Critical Hdwy	6.48	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.48	-	-	-	-	-
Critical Hdwy Stg 2	5.48	-	-	-	-	-
Follow-up Hdwy	3.572	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	242	608	-	-	1083	-
Stage 1	630	-	-	-	-	-
Stage 2	531	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	235	605	_	-	1078	_
Mov Cap-2 Maneuver	365	-	-	_	-	-
Stage 1	627	_	-	_	-	_
Stage 2	519	_	_	-	_	_
0.000 2	010					
	\4/D				0.5	
Approach	WB		NB		SB	
HCM Control Delay, s	13		0		0.3	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1V	VBLn2	SBL
Capacity (veh/h)		_	_		605	1078
HCM Lane V/C Ratio		_	_	0.043		
HCM Control Delay (s)		_	_		11.1	8.4
HCM Lane LOS		_	_	C	В	A
HCM 95th %tile Q(veh)		-	-	0.1	0.1	0.1

Lanes, Volumes, Timings 2: Sandwich Street South & Parking Lot Driveway

	•	•	†	/	>	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1>		ሻ	†
Traffic Volume (vph)	3	23	426	34	16	532
Future Volume (vph)	3	23	426	34	16	532
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0		0.0	15.0	
Storage Lanes	1	0		0	1	
Taper Length (m)	15.0				15.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.880		0.990			
Flt Protected	0.994				0.950	
Satd. Flow (prot)	1611	0	1807	0	1750	1807
Flt Permitted	0.994				0.950	
Satd. Flow (perm)	1611	0	1807	0	1750	1807
Link Speed (k/h)	30		50			50
Link Distance (m)	133.6		15.2			44.2
Travel Time (s)	16.0		1.1			3.2
Confl. Peds. (#/hr)				19	19	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	2%	2%	3%	2%	2%	4%
Adj. Flow (vph)	3	24	444	35	17	554
Shared Lane Traffic (%)						
Lane Group Flow (vph)	27	0	479	0	17	554
Enter Blocked Intersection	No	No	Yes	No	No	Yes
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		3.5			3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	15		15	25	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type: (Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 38.0%			IC	CU Level o	of Service A
Analysis Davidal (min) 15						

Analysis Period (min) 15

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131 & 135 Sandwich Street South

Intersection						
Int Delay, s/veh	0.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		î,		*	†
Traffic Vol, veh/h	3	23	426	34	16	532
Future Vol, veh/h	3	23	426	34	16	532
Conflicting Peds, #/hr	0	0	0	19	19	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	96	96	96	96	96	96
Heavy Vehicles, %	2	2	3	2	2	4
Mvmt Flow	3	24	444	35	17	554
Major/Mina-	die - 4		lois -1		10:5-0	
	Minor1		Major1		Major2	^
Conflicting Flow All	1069	481	0	0	498	0
Stage 1	481	-	-	-	-	-
Stage 2	588	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-		-
Follow-up Hdwy		3.318	-	-	2.218	-
Pot Cap-1 Maneuver	245	585	-	-	1066	-
Stage 1	622	-	-	-	-	-
Stage 2	555	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	237	575	-	-	1047	-
Mov Cap-2 Maneuver	371	-	-	-	-	-
Stage 1	611	-	-	-	-	-
Stage 2	546	-	-	-	-	-
A	1A/D		NID		0.0	
Approach	WB		NB		SB	
HCM Control Delay, s	12		0		0.2	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)			-	541	1047	-
HCM Lane V/C Ratio		_	-		0.016	<u>-</u>
HCM Control Delay (s)		-		12	8.5	_
HCM Lane LOS		-	-	B	6.5 A	-
HCM 95th %tile Q(veh)	1	-		0.2	0	
	1		-	U.Z	U	-

131 & 135 Sandwich Street South

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			44		ሻ	f)		7	₽	
Traffic Volume (vph)	4	0	18	2	0	0	62	451	5	7	508	29
Future Volume (vph)	4	0	18	2	0	0	62	451	5	7	508	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0		0.0	0.0		0.0	10.0		0.0	30.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.887						0.998			0.992	
Flt Protected		0.992			0.950		0.950			0.950		
Satd. Flow (prot)	0	1459	0	0	1750	0	1653	1821	0	1750	1776	0
Flt Permitted		0.992			0.950		0.950			0.950		
Satd. Flow (perm)	0	1459	0	0	1750	0	1653	1821	0	1750	1776	0
Link Speed (k/h)		50			30			50			50	
Link Distance (m)		127.1			58.9			86.3			107.9	
Travel Time (s)		9.2			7.1			6.2			7.8	
Confl. Peds. (#/hr)			1	1			9		26	26		9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	8%	3%	2%	2%	5%	4%
Parking (#/hr)		0										
Adj. Flow (vph)	4	0	20	2	0	0	69	501	6	8	564	32
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	24	0	0	2	0	69	507	0	8	596	0
Enter Blocked Intersection	No	No	No	No	No	No	No	Yes	No	No	Yes	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0	•		0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.01	1.16	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
	Other											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 45.6%			IC	CU Level	of Service	eΑ					
A1 - '- D - '- 1 / - '-\ 4 F												

Analysis Period (min) 15

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131 & 135 Sandwich Street South

Intersection												
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ች	1→		ሻ	ĵ.	
Traffic Vol, veh/h	4	0	18	2	0	0	62	451	5	7	508	29
Future Vol, veh/h	4	0	18	2	0	0	62	451	5	7	508	29
Conflicting Peds, #/hr	0	0	1	1	0	0	9	0	26	26	0	9
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	300	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	8	3	2	2	5	4
Mvmt Flow	4	0	20	2	0	0	69	501	6	8	564	32
Major/Minor	Minor2			Minor1			Major1		ľ	Major2		
Conflicting Flow All	1247	1276	590	1275	1289	530	605	0	0	533	0	0
Stage 1	605	605	-	668	668	-	-	-	-	-	-	-
Stage 2	642	671	-	607	621	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.18	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.272	-	-	2.218	-	-
Pot Cap-1 Maneuver	150	167	508	144	164	549	944	-	-	1035	-	-
Stage 1	485	487	-	448	456	-	-	-	-	-	-	-
Stage 2	463	455	-	483	479	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	140	148	503	126	146	536	936	-	-	1010	-	-
Mov Cap-2 Maneuver	140	148	-	126	146	-	-	-	-	-	-	-
Stage 1	446	479	-	405	412	-	-	-	-	-	-	-
Stage 2	429	411	-	460	471	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	16.3			34.1			1.1			0.1		
HCM LOS	С			D								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		936	_	-	342	126	1010	_	_			
HCM Lane V/C Ratio		0.074	-	_	0.071			_	-			
HCM Control Delay (s)		9.2	-	-	16.3	34.1	8.6	-	-			
HCM Lane LOS		A	-	-	С	D	A	-	-			
HCM 95th %tile Q(veh)	0.2	-	-	0.2	0.1	0	-	-			

	۶	→	•	•	—	4	4	†	~	\	+	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations											†	
Traffic Volume (vph)	0	0	0	0	0	0	0	460	0	0	535	0
Future Volume (vph)	0	0	0	0	0	0	0	460	0	0	535	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt												
Flt Protected												
Satd. Flow (prot)	0	0	0	0	0	0	0	1842	0	0	1842	0
Flt Permitted		•			· ·			1012		•	1012	J
Satd. Flow (perm)	0	0	0	0	0	0	0	1842	0	0	1842	0
Right Turn on Red	•	· ·	Yes	· ·	· ·	Yes	J	1012	Yes	•	1012	Yes
Satd. Flow (RTOR)			. 00			. 00			. 00			. 00
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		27.0			26.6			14.0			22.4	
Travel Time (s)		1.9			1.9			1.0			1.6	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	0.50	0.30	0.50	0.50	0.50	0.30	0.30	479	0.50	0.50	557	0.50
Shared Lane Traffic (%)	<u> </u>						, ,	47.5		U	001	J
Lane Group Flow (vph)	0	0	0	0	0	0	0	479	0	0	557	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	LOIL	0.0	rtigrit	LOIL	0.0	ragnt	LOIL	3.5	rtigiit	LOIL	3.5	rtigrit
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane		0.0			0.0			Yes			Yes	
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	1.01	15	25	1.01	15	25	1.01	15	25	1.01	15
Number of Detectors	20		10	20		10	20	2	10	20	2	10
Detector Template								Thru			Thru	
Leading Detector (m)								10.0			10.0	
Trailing Detector (m)								0.0			0.0	
Detector 1 Position(m)								0.0			0.0	
Detector 1 Size(m)								0.6			0.6	
Detector 1 Type								CI+Ex			CI+Ex	
Detector 1 Channel								OITEX			OIILX	
Detector 1 Extend (s)								0.0			0.0	
Detector 1 Queue (s)								0.0			0.0	
Detector 1 Delay (s)								0.0			0.0	
Detector 2 Position(m)								9.4			9.4	
Detector 2 Size(m)								0.6			0.6	
Detector 2 Type								CI+Ex			CI+Ex	
Detector 2 Type Detector 2 Channel								OITEX			OIILX	
Detector 2 Extend (s)								0.0			0.0	
Turn Type								NA			NA	
Protected Phases								2			6	
Permitted Phases											U	
Detector Phase								2			6	
Switch Phase											U	
Minimum Initial (s)								10.0			10.0	
iviii iiiiiiiiiiiiiii (S)								10.0			10.0	

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Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Peak Hour Factor	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h) Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	40.0
Minimum Initial (s)	10.0

	۶	→	•	•	←	•	•	†	<i>></i>	/	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)								24.5			24.5	
Total Split (s)								30.0			30.0	
Total Split (%)								50.0%			50.0%	
Maximum Green (s)								23.5			23.5	
Yellow Time (s)								3.3			3.3	
All-Red Time (s)								3.2			3.2	
Lost Time Adjust (s)								0.0			0.0	
Total Lost Time (s)								6.5			6.5	
Lead/Lag								0.0			0.0	
Lead-Lag Optimize?												
Vehicle Extension (s)								3.0			3.0	
Recall Mode								Max			Max	
Walk Time (s)								7.0			7.0	
Flash Dont Walk (s)								11.0			11.0	
Pedestrian Calls (#/hr)								0			0	
								42.1			42.1	
Act Effct Green (s)								0.61			0.61	
Actuated g/C Ratio												
v/c Ratio								0.43			0.50	
Control Delay								12.6			13.7	
Queue Delay								0.0			0.0	
Total Delay								12.6			13.7	
LOS								В			В	
Approach Delay								12.6			13.7	
Approach LOS								В			В	
Queue Length 50th (m)								41.7			51.4	
Queue Length 95th (m)								65.5			80.2	
Internal Link Dist (m)		3.0			2.6			0.1			0.1	
Turn Bay Length (m)												
Base Capacity (vph)								1124			1124	
Starvation Cap Reductn								0			0	
Spillback Cap Reductn								0			0	
Storage Cap Reductn								0			0	
Reduced v/c Ratio								0.43			0.50	
Intersection Summary												
	Other											
Cycle Length: 60	Outo											
Actuated Cycle Length: 69												
Natural Cycle: 60												
Control Type: Semi Act-Und	roord											
Maximum v/c Ratio: 0.50	,001 u											
Intersection Signal Delay: 1	3 2			In	itersection	1 OS: B						
Intersection Capacity Utiliza						of Service	۸					
Analysis Period (min) 15	111011 30.2 /0			IC.	O Level	JI SEIVICE	^					
Splits and Phases: 4: Sar	ndwich Stre	eet South	& Pedest	rian Cros	ÅÅ	0 4						
30 s					30 s							
₩ Ø6					- 1							
30 s												

Page 9 RM

Lane Group	Ø4
Minimum Split (s)	30.0
Total Split (s)	30.0
Total Split (%)	50%
Maximum Green (s)	26.0
Yellow Time (s)	3.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	19.0
Pedestrian Calls (#/hr)	137
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

	•	4	†	1	-	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		^			†
Traffic Volume (vph)	0	0	460	0	0	535
Future Volume (vph)	0	0	460	0	0	535
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						
Flt Protected						
Satd. Flow (prot)	1842	0	1842	0	0	1842
Flt Permitted						
Satd. Flow (perm)	1842	0	1842	0	0	1842
Link Speed (k/h)	30		50			50
Link Distance (m)	44.7		22.4			15.2
Travel Time (s)	5.4		1.6			1.1
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	0	0	479	0	0	557
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	479	0	0	557
Enter Blocked Intersection	No	No	Yes	No	No	Yes
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		3.5			3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	15		15	25	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type: Ot	her					
Control Type: Unsignalized						
Intersection Capacity Utilizatio	n 38.2%			IC	U Level c	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	VVDIX	<u> </u>	NDIX	ODL	<u> </u>
Traffic Vol, veh/h	0	0	460	0	0	535
Future Vol, veh/h	0	0	460	0	0	535
	0	0		0	0	
Conflicting Peds, #/hr			0			0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	96	96	96	96	96	96
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	479	0	0	557
	Minor1		Major1	N	/lajor2	
Conflicting Flow All	1036	479	0	-	-	-
Stage 1	479	-	-	-	-	-
Stage 2	557	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy		3.318	_	_	_	_
Pot Cap-1 Maneuver	256	587	_	0	0	_
Stage 1	623	-	_	0	0	_
Stage 2	574			0	0	_
Platoon blocked, %					U	_
Platoon blocked %	317	-	-	U		
			-			-
Mov Cap-1 Maneuver	256	587		-	-	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver	256 389		-		-	
Mov Cap-1 Maneuver	256 389 623	587	-	-		-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver	256 389	587 -	- - -	-	-	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1	256 389 623	587 - -	- - -	-	-	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2	256 389 623 574	587 - -	- - - -	-	- - -	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2	256 389 623 574 WB	587 - -	- - - - NB	-	- - - SB	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s	256 389 623 574 WB	587 - -	- - - -	-	- - -	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2	256 389 623 574 WB	587 - -	- - - - NB	-	- - - SB	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s	256 389 623 574 WB	587 - -	- - - - NB	-	- - - SB	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS	256 389 623 574 WB 0 A	587 - - -	- - - - - NB 0	-	- - - SB	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvn	256 389 623 574 WB 0 A	587 - - -	- - - - NB	-	- - - SB	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h)	256 389 623 574 WB 0 A	587 - - - - NBTW	- - - - NB 0	SBT	- - - SB	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	256 389 623 574 WB 0 A	587 - - - - - NBTW - -	- - - - NB 0	SBT	- - - SB	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	256 389 623 574 WB 0 A	587 - - - - - NBTW - -	- - - - - 0	- - - - SBT - -	- - - SB	-
Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	256 389 623 574 WB 0 A	587 - - - - - NBTW - -	- - - - NB 0	SBT	- - - SB	-

6: Sandwich Street South & Site Access #2

	•	•	†	/	>	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1}•		ሻ	†
Traffic Volume (vph)	1	1	459	1	1	534
Future Volume (vph)	1	1	459	1	1	534
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0		0.0	5.0	
Storage Lanes	1	0		0	1	
Taper Length (m)	15.0				15.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.932					
Flt Protected	0.976				0.950	
Satd. Flow (prot)	1676	0	1842	0	1750	1842
Flt Permitted	0.976				0.950	
Satd. Flow (perm)	1676	0	1842	0	1750	1842
Link Speed (k/h)	30		50			50
Link Distance (m)	35.1		107.9			14.0
Travel Time (s)	4.2		7.8			1.0
Confl. Peds. (#/hr)				5	5	
Confl. Bikes (#/hr)				5		
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	1	1	478	1	1	556
Shared Lane Traffic (%)						
Lane Group Flow (vph)	2	0	479	0	1	556
Enter Blocked Intersection	No	No	Yes	No	No	Yes
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		3.5			3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	15		15	25	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 38.1%			IC	CU Level o	of Service A
Analysis Davis d (mis) 15						

Analysis Period (min) 15

131 & 135 Sandwich Street South

0 WBL	WBR	NBT	MDD		
	WBR	NRT	NDD		
			NBR	SBL	SBT
		1	TI DIT	<u> </u>	<u> </u>
1	1	459	1	1	534
1	1	459	1	1	534
0	0	0	5	5	0
Stop	Stop	Free	Free	Free	Free
					None
					-
					0
					0
					96
					2
1	1	478	1	1	556
Minor1	Λ	Maior1	1	Maior2	
					0
					-
				_	
					-
				4.12	-
			-	-	-
	-	-	-	-	-
		-	-		-
	583	-	-	1079	-
	-	-	-	-	-
573	-	-	-	-	-
		-	-		-
252	580	-	-	1074	-
	-	-	_	-	-
	_	_	-	_	_
		_	_	_	_
JIZ				_	
WB		NB		SB	
12.8		0		0	
nt	NBT	NBRV			SBT
	-	-	464	1074	-
	-	-	0.004	0.001	-
	-	-			-
	-	-	В	A	-
)	_	_	0	0	_
		- None 0 9, # 0 96 96 2 2 1 1 Minor1 N 1042 484 484 558 6.42 6.22 5.42 3.518 3.318 254 583 620 573 252 580 386 617 572 WB 12.8 B nt NBT	- None - 0 e, # 0 - 0 96 96 96 2 2 2 2 1 1 478 Minor1 Major1 1042 484 0 484 558 6.42 6.22 - 5.42 3.518 3.318 - 254 583 - 620 573 252 580 - 386 617 572 WB NB 12.8 0 B	- None - None 0 9, # 0 - 0 - 0 - 0 - 96 96 96 96 2 2 2 2 2 1 1 478 1 Minor1	- None - None - 0 50 e, # 0 - 0 96 96 96 96 96 2 2 2 2 2 2 1 1 478 1 1 Minor1

Lanes, Volumes, Timings 1: Sandwich Street South & Fort Street

	•	•	†	/	\	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	7	^		ሻ	†
Traffic Volume (vph)	44	26	658	83	14	675
Future Volume (vph)	44	26	658	83	14	675
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	20.0	0.0		0.0	30.0	
Storage Lanes	1	1		0	1	
Taper Length (m)	15.0				15.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt		0.850	0.985			
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1750	1566	1814	0	1750	1842
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1750	1566	1814	0	1750	1842
Link Speed (k/h)	40		50			50
Link Distance (m)	238.1		44.2			71.5
Travel Time (s)	21.4		3.2			5.1
Confl. Peds. (#/hr)	1			5	5	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	45	27	678	86	14	696
Shared Lane Traffic (%)						
Lane Group Flow (vph)	45	27	764	0	14	696
Enter Blocked Intersection	No	No	Yes	No	No	Yes
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		3.5			3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	15		15	25	
Sign Control	Stop		Free			Free
Intersection Summary						
· · · · · · · · · · · · · · · · · · ·	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 49.7%			IC	CU Level o	of Service A
Analysis Period (min) 15						, , , , , , ,

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Intersection						
Int Delay, s/veh	0.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	ች	7	f		ች	
Traffic Vol, veh/h	44	26	658	83	14	675
Future Vol, veh/h	44	26	658	83	14	675
Conflicting Peds, #/hr	1	0	0	5	5	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	200	0	_	-	300	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	45	27	678	86	14	696
Miller 1011		=!	0.0		•	000
	4					
	Minor1		Major1		Major2	_
Conflicting Flow All	1451	726	0	0	769	0
Stage 1	726	-	-	-	-	-
Stage 2	725	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518		-	-	2.218	-
Pot Cap-1 Maneuver	144	425	-	-	845	-
Stage 1	479	-	-	-	-	-
Stage 2	479	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	141	423	-	-	841	-
Mov Cap-2 Maneuver	280	-	-	-	-	-
Stage 1	477	-	-	-	-	-
Stage 2	470	-	-	-	-	-
Annragah	WD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	18		0		0.2	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1V	VBLn2	SBL
Capacity (veh/h)		_	_	280	423	841
HCM Lane V/C Ratio		_	-	0.162		
HCM Control Delay (s)		-	_	20.3	14.1	9.4
HCM Lane LOS		_	-	С	В	Α
HCM 95th %tile Q(veh))	-	-	0.6	0.2	0.1

2: Sandwich Street South & Parking Lot Driveway

	•	•	†	/	>	ļ	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	¥		ĵ.		7	†	
Traffic Volume (vph)	12	11	728	16	7	709	
Future Volume (vph)	12	11	728	16	7	709	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (m)	0.0	0.0		0.0	15.0		
Storage Lanes	1	0		0	1		
Taper Length (m)	15.0				15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor							
Frt	0.935		0.997				
Flt Protected	0.975				0.950		
Satd. Flow (prot)	1679	0	1837	0	1750	1842	
Flt Permitted	0.975				0.950		
Satd. Flow (perm)	1679	0	1837	0	1750	1842	
Link Speed (k/h)	30		50			50	
Link Distance (m)	133.6		15.0			44.2	
Travel Time (s)	16.0		1.1			3.2	
Confl. Peds. (#/hr)	4	1		4	4		
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	
Adj. Flow (vph)	12	11	743	16	7	723	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	23	0	759	0	7	723	
Enter Blocked Intersection	No	No	Yes	No	No	Yes	
Lane Alignment	Left	Right	Left	Right	Left	Left	
Median Width(m)	3.5		3.5			3.5	
Link Offset(m)	0.0		0.0			0.0	
Crosswalk Width(m)	3.0		3.0			3.0	
Two way Left Turn Lane			Yes			Yes	
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01	
Turning Speed (k/h)	25	15		15	25		
Sign Control	Stop		Free			Free	
Intersection Summary							
71	Other						
Control Type: Unsignalized							
Intersection Canacity Litiliza	tion 10 6%			IC	ا المرادات	of Service A	Δ

Intersection Capacity Utilization 49.6%

ICU Level of Service A

Analysis Period (min) 15

2023 FT PM

131 & 135 Sandwich Street South

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		\$		<u> </u>	<u> </u>
Traffic Vol, veh/h	12	11	728	16	7	709
Future Vol, veh/h	12	11	728	16	7	709
Conflicting Peds, #/hr	4	1	0	4	4	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	_	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage		-	0	_	-	0
Grade, %	0	_	0	-	_	0
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	11	743	16	7	723
With the William		• •	1 10		•	. 20
	Minor1		Major1		Major2	
Conflicting Flow All	1496	756	0	0	763	0
Stage 1	755	-	-	-	-	-
Stage 2	741	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	135	408	-	-	850	-
Stage 1	464	-	-	-	-	-
Stage 2	471	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	133	406	-	-	847	-
Mov Cap-2 Maneuver	272	-	-	-	-	-
Stage 1	462	-	-	-	-	-
Stage 2	465	-	-	-	-	-
Annraaah	WB		ND		SB	
Approach			NB			
HCM Control Delay, s			0		0.1	
HCM LOS	С					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_	_	323	847	_
HCM Lane V/C Ratio		_	_	0.073		_
)	_	_	17	9.3	_
HCM Control Delay (s	/			C	Α	_
HCM Control Delay (s HCM Lane LOS		-	-	()		
HCM Control Delay (s HCM Lane LOS HCM 95th %tile Q(veh	1)	-	-	0.2	0	_

131 & 135 Sandwich Street South

	۶	→	•	•	←	4	1	†	~	/		4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	f)		Ţ	£	
Traffic Volume (vph)	5	0	5	4	0	4	4	738	0	4	712	8
Future Volume (vph)	5	0	5	4	0	4	4	738	0	4	712	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0		0.0	0.0		0.0	10.0		0.0	30.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.932			0.932						0.998	
Flt Protected		0.976			0.976		0.950			0.950		
Satd. Flow (prot)	0	1508	0	0	1676	0	1750	1842	0	1750	1838	0
Flt Permitted		0.976			0.976		0.950			0.950		
Satd. Flow (perm)	0	1508	0	0	1676	0	1750	1842	0	1750	1838	0
Link Speed (k/h)		50			30			50			50	
Link Distance (m)		127.1			58.9			86.3			107.9	
Travel Time (s)		9.2			7.1			6.2			7.8	
Confl. Peds. (#/hr)							8		1	1		8
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Parking (#/hr)		0										
Adj. Flow (vph)	5	0	5	4	0	4	4	753	0	4	727	8
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	10	0	0	8	0	4	753	0	4	735	0
Enter Blocked Intersection	No	No	No	No	No	No	No	Yes	No	No	Yes	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.01	1.16	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Stop			Stop			Free			Free	
Intonocation Commence												

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 48.8%

ICU Level of Service A

Analysis Period (min) 15

HCM 2010 TWSC 2023 FT PM

131 & 135 Sandwich Street South

Intersection												
Int Delay, s/veh	0.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	f)		ሻ	(î	
Traffic Vol, veh/h	5	0	5	4	0	4	4	738	0	4	712	8
Future Vol, veh/h	5	0	5	4	0	4	4	738	0	4	712	8
Conflicting Peds, #/hr	0	0	0	0	0	0	8	0	1	1	0	8
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	300	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	98	98	98	98	98	98	98	98	98	98	98	98
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	5	0	5	4	0	4	4	753	0	4	727	8
Major/Minor I	Minor2			Minor1			Major1		N	//ajor2		
Conflicting Flow All	1510	1509	739	1504	1513	754	743	0	0	754	0	0
Stage 1	747	747	-	762	762	-	-	-	-	-	_	-
Stage 2	763	762	-	742	751	-	_	_	-	-	_	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	_	_	4.12	_	_
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	_	_	2.218	-	_
Pot Cap-1 Maneuver	99	120	417	100	120	409	864	-	-	856	-	-
Stage 1	405	420	-	397	414	-	-	-	-	-	-	-
Stage 2	397	414	-	408	418	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	97	118	414	98	118	409	858	-	-	855	-	-
Mov Cap-2 Maneuver	97	118	-	98	118	-	-	-	-	-	-	-
Stage 1	400	415	-	395	412	-	-	-	-	-	-	-
Stage 2	391	412	-	401	413	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	29.5			29			0			0.1		
HCM LOS	23.5 D			D			U			0.1		
TOW LOO	J			U								
							0.51	05-	055			
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V		SBL	SBT	SBR			
Capacity (veh/h)		858	-	-	157	158	855	-	-			
HCM Lane V/C Ratio		0.005	-	-	0.065			-	-			
HCM Control Delay (s)		9.2	-	-	29.5	29	9.2	-	-			
HCM Lane LOS		Α	-	-	D	D	Α	-	-			
HCM 95th %tile Q(veh)	0	-	-	0.2	0.2	0	-	-			

4: Gariawich Street	Codin	x 1 0 u c	Journali	0.000	n ig							
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations								^			^	
Traffic Volume (vph)	0	0	0	0	0	0	0	744	0	0	721	0
Future Volume (vph)	0	0	0	0	0	0	0	744	0	0	721	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt												
Flt Protected												
Satd. Flow (prot)	0	0	0	0	0	0	0	1842	0	0	1842	0
Flt Permitted	•	•	•	•	•					•		_
Satd. Flow (perm)	0	0	0	0	0	0	0	1842	0	0	1842	0
Right Turn on Red	· ·	V	Yes	J	•	Yes	J	1012	Yes		1012	Yes
Satd. Flow (RTOR)			100			100			100			100
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		31.3			34.9			16.4			20.2	
Travel Time (s)		2.3			2.5			1.2			1.5	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	0.30	0.30	0.30	0.30	0.30	0.30	0.30	759	0.30	0.30	736	0.30
Shared Lane Traffic (%)	U	U	U	U	U	U	U	133	U	U	7 30	U
Lane Group Flow (vph)	0	0	0	0	0	0	0	759	0	0	736	0
Enter Blocked Intersection			No	No		No	No	No	No			
	No	No			No					No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.5			3.5	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane	4.04	4.04	4.04	4.04	4.04	4.04	4.04	Yes	4.04	4.04	Yes	4.04
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25		15	25		15	25	•	15	25	•	15
Number of Detectors								2			2	
Detector Template								Thru			Thru	
Leading Detector (m)								10.0			10.0	
Trailing Detector (m)								0.0			0.0	
Detector 1 Position(m)								0.0			0.0	
Detector 1 Size(m)								0.6			0.6	
Detector 1 Type								CI+Ex			CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)								0.0			0.0	
Detector 1 Queue (s)								0.0			0.0	
Detector 1 Delay (s)								0.0			0.0	
Detector 2 Position(m)								9.4			9.4	
Detector 2 Size(m)								0.6			0.6	
Detector 2 Type								CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)								0.0			0.0	
Turn Type								NA			NA	
Protected Phases								2			6	
Permitted Phases												
Detector Phase								2			6	
Switch Phase												
Minimum Initial (s)								10.0			10.0	
· · · · · · · · · · · · · · · · · · ·												

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Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (k/h)	
Link Distance (m)	
Travel Time (s)	
Peak Hour Factor	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(m)	
Link Offset(m)	
Crosswalk Width(m)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (k/h)	
Number of Detectors	
Detector Template	
Leading Detector (m)	
Trailing Detector (m)	
Detector 1 Position(m)	
Detector 1 Size(m)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(m)	
Detector 2 Size(m)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)								32.5			24.5	
Total Split (s)								35.0			35.0	
Total Split (%)								53.8%			53.8%	
Maximum Green (s)								28.5			28.5	
Yellow Time (s)								3.3			3.3	
All-Red Time (s)								3.2			3.2	
Lost Time Adjust (s)								0.0			0.0	
Total Lost Time (s)								6.5			6.5	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)								3.0			3.0	
Recall Mode								Max			Max	
Walk Time (s)								7.0			7.0	
Flash Dont Walk (s)								11.0			11.0	
Pedestrian Calls (#/hr)								0			0	
Act Effct Green (s)								50.9			50.9	
Actuated g/C Ratio								0.91			0.91	
								0.91				
v/c Ratio											0.44	
Control Delay								5.7			5.5	
Queue Delay								0.0			0.0	
Total Delay								5.7			5.5	
LOS								A			Α	
Approach Delay								5.7			5.5	
Approach LOS								Α			Α	
Queue Length 50th (m)								0.0			0.0	
Queue Length 95th (m)								128.4			121.5	
Internal Link Dist (m)		7.3			10.9			0.1			0.1	
Turn Bay Length (m)												
Base Capacity (vph)								1674			1674	
Starvation Cap Reductn								0			0	
Spillback Cap Reductn								0			0	
Storage Cap Reductn								0			0	
Reduced v/c Ratio								0.45			0.44	
Intersection Summary												
Area Type:	Other											
Cycle Length: 65												
Actuated Cycle Length: 56												
Natural Cycle: 70												
Control Type: Semi Act-Und	coord											
Maximum v/c Ratio: 0.45												
Intersection Signal Delay: 5	5.6			In	tersection	LOS: A						
Intersection Capacity Utiliza						of Service	Α					
Analysis Period (min) 15												
Splits and Phases: 4: Sa	ndwich Stre	et South	& Pedest	rian Cros	sina							
*		554411	04001	0100		₹k _{Ø4}						
Ø2 35 s						л №04 30 s						
1												
▼ Ø6 35 s												

Minimum Split (s) 30.0 Total Split (s) 30.0 Total Split (%) 46% Maximum Green (s) 26.0 Yellow Time (s) 3.0 All-Red Time (s) 1.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) 3.0 Recall Mode None Walk Time (s) 7.0 Flash Dont Walk (s) 19.0 Pedestrian Calls (#/hr) 4 Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio Intersection Summary	Lane Group	Ø4
Total Split (s) 30.0 Total Split (%) 46% Maximum Green (s) 26.0 Yellow Time (s) 3.0 All-Red Time (s) 1.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) 3.0 Recall Mode None Walk Time (s) 7.0 Flash Dont Walk (s) 19.0 Pedestrian Calls (#/hr) 4 Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Total Split (%) 46% Maximum Green (s) 26.0 Yellow Time (s) 3.0 All-Red Time (s) 1.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) 3.0 Recall Mode None Walk Time (s) 7.0 Flash Dont Walk (s) 19.0 Pedestrian Calls (#/hr) 4 Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Maximum Green (s) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) Recall Mode Walk Time (s) Flash Dont Walk (s) Pedestrian Calls (#/hr) Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Yellow Time (s) 3.0 All-Red Time (s) 1.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) 3.0 Recall Mode None Walk Time (s) 7.0 Flash Dont Walk (s) 19.0 Pedestrian Calls (#/hr) 4 Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) Recall Mode Walk Time (s) Flash Dont Walk (s) Pedestrian Calls (#/hr) Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) 3.0 Recall Mode None Walk Time (s) 7.0 Flash Dont Walk (s) 19.0 Pedestrian Calls (#/hr) 4 Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) Recall Mode Walk Time (s) Flash Dont Walk (s) Pedestrian Calls (#/hr) Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		1.0
Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) 3.0 Recall Mode None Walk Time (s) 7.0 Flash Dont Walk (s) 19.0 Pedestrian Calls (#/hr) 4 Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Lead-Lag Optimize? Vehicle Extension (s) 3.0 Recall Mode None Walk Time (s) 7.0 Flash Dont Walk (s) 19.0 Pedestrian Calls (#/hr) 4 Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Vehicle Extension (s) Recall Mode Walk Time (s) Flash Dont Walk (s) Pedestrian Calls (#/hr) Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		
Recall Mode Walk Time (s) Flash Dont Walk (s) Pedestrian Calls (#/hr) Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		2.0
Walk Time (s) 7.0 Flash Dont Walk (s) 19.0 Pedestrian Calls (#/hr) 4 Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Flash Dont Walk (s) Pedestrian Calls (#/hr) Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Pedestrian Calls (#/hr) 4 Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		4
v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Queue Length 95th (m)	
Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Internal Link Dist (m)	
Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		
Storage Cap Reductn Reduced v/c Ratio		
Reduced v/c Ratio		
Intersection Summary		
intersection Summary	Interpostion Commen	
	intersection Summary	

	•	4	†	~	-	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		^			†
Traffic Volume (vph)	8	7	737	0	0	721
Future Volume (vph)	8	7	737	0	0	721
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.937					
Flt Protected	0.974					
Satd. Flow (prot)	1681	0	1842	0	0	1842
Flt Permitted	0.974					
Satd. Flow (perm)	1681	0	1842	0	0	1842
Link Speed (k/h)	30		50			50
Link Distance (m)	45.8		20.2			15.0
Travel Time (s)	5.5		1.5			1.1
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	8	7	752	0	0	736
Shared Lane Traffic (%)						
Lane Group Flow (vph)	15	0	752	0	0	736
Enter Blocked Intersection	No	No	Yes	No	No	Yes
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		3.5			3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	15		15	25	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type: C	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizati	on 48.8%			IC	U Level o	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W					<u> </u>
Traffic Vol, veh/h	8	7	737	0	0	721
Future Vol, veh/h	8	7	737	0	0	721
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		-	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	8	7	752	0	0	736
IVIVIII(I IOW	U	1	102	U	U	700
	Minor1		Major1	N	/lajor2	
Conflicting Flow All	1488	752	0	-	-	-
Stage 1	752	-	-	-	-	-
Stage 2	736	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	-	-
Pot Cap-1 Maneuver	137	410	-	0	0	-
Stage 1	466	-	-	0	0	-
Stage 2	474	-	-	0	0	-
Platoon blocked, %			-			-
Mov Cap-1 Maneuver	137	410	-	-	-	-
Mov Cap-2 Maneuver	277	-	-	-	-	-
Stage 1	466	-	-	-	-	-
Stage 2	474	_	-	_	_	-
5 3 5						
A Iv	\A/D		ND		00	
Approach	WB		NB		SB	
HCM Control Delay, s	16.6		0		0	
HCM LOS	С					
Minor Lane/Major Mvn	nt	NBTV	VBLn1	SBT		
Capacity (veh/h)		-		-		
HCM Lane V/C Ratio			0.047	_		
HCM Control Delay (s	\	_		_		
)					
HOW COM /OMIC Q(VCI)	7		V. 1			
HCM Lane LOS HCM 95th %tile Q(veh		-	C 0.1	-		

Analysis Period (min) 15

	•	•	†	/	/	ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĵ.		7	†
Traffic Volume (vph)	5	5	732	14	13	716
Future Volume (vph)	5	5	732	14	13	716
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0		0.0	5.0	
Storage Lanes	1	0		0	1	
Taper Length (m)	15.0				15.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.932		0.998			
Flt Protected	0.976				0.950	
Satd. Flow (prot)	1676	0	1838	0	1750	1842
Flt Permitted	0.976				0.950	
Satd. Flow (perm)	1676	0	1838	0	1750	1842
Link Speed (k/h)	30		50			50
Link Distance (m)	52.1		107.9			16.4
Travel Time (s)	6.3		7.8			1.2
Confl. Peds. (#/hr)				5	5	
Confl. Bikes (#/hr)				5		
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	5	5	747	14	13	731
Shared Lane Traffic (%)						
Lane Group Flow (vph)	10	0	761	0	13	731
Enter Blocked Intersection	No	No	Yes	No	No	Yes
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.5		3.5			3.5
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.01	1.01	1.01	1.01	1.01	1.01
Turning Speed (k/h)	25	15		15	25	
Sign Control	Stop		Free			Free
Intersection Summary						
Jr -	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	tion 49.4%			IC	CU Level c	of Service
A 1 - 1 - D 1 - 1 / - 1 - \ 45						

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL.	וטייי	1\D1	אפא	JDL Š	<u> </u>
Traffic Vol, veh/h	5	5	732	14	13	716
Future Vol, veh/h	5	5	732	14	13	716
Conflicting Peds, #/hr	0	0	0	5	5	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- Clop	None	-		-	None
Storage Length	0	-	_	-	50	-
Veh in Median Storage		_	0	_	-	0
Grade, %	0	<u>-</u>	0	<u>-</u>	_	0
Peak Hour Factor	98	98	98	98	98	98
	2	2	2	2	2	2
Heavy Vehicles, %	5	5	747	14	13	
Mvmt Flow	5	5	141	14	13	731
Major/Minor I	Minor1	N	Major1	N	Major2	
Conflicting Flow All	1516	759	0	0	766	0
Stage 1	759	-	-	-	-	-
Stage 2	757	-	-	-	-	-
Critical Hdwy	6.42	6.22	_	_	4.12	_
Critical Hdwy Stg 1	5.42	-	-	_	_	_
Critical Hdwy Stg 2	5.42	_	-	_	-	-
Follow-up Hdwy	3.518	3.318	_	_	2.218	_
Pot Cap-1 Maneuver	131	406	_	_	847	_
Stage 1	462	-	_	_	-	_
Stage 2	463	_	_	_	_	_
Platoon blocked, %	400		_	_		_
Mov Cap-1 Maneuver	128	404	_	_	843	_
Mov Cap-1 Maneuver	267	-	_	_	0+0	_
Stage 1	460	-	-	_	_	
_	456	<u>-</u>	_	_		_
Stage 2	430	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	16.5		0		0.2	
HCM LOS	С					
Min and an a/Mailen Manne	.1	NDT	NDDV	VDI 4	CDI	CDT
Minor Lane/Major Mvm)Ţ	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-	-		843	-
HCM Lane V/C Ratio		-		0.032		-
HCM Control Delay (s)		-	-		9.3	-
HCM Lane LOS		-	-	С	Α	-
HCM 95th %tile Q(veh))	-	-	0.1	0	-

Intersection: 1: Sandwich Street South & Fort Street

Movement	WB	WB	NB	SB	SB
Directions Served	L	R	TR	L	Т
Maximum Queue (m)	18.4	9.0	13.0	12.0	12.6
Average Queue (m)	4.5	3.8	0.4	2.3	1.0
95th Queue (m)	13.5	11.0	7.2	9.3	8.0
Link Distance (m)		228.3	30.4		61.8
Upstream Blk Time (%)			0		
Queuing Penalty (veh)			0		
Storage Bay Dist (m)	20.0			30.0	
Storage Blk Time (%)	1				0
Queuing Penalty (veh)	0				0

Intersection: 2: Sandwich Street South & Parking Lot Driveway

Movement	WB	NB	SB	SB
Directions Served	LR	TR	L	T
Maximum Queue (m)	12.9	6.8	18.8	48.8
Average Queue (m)	4.9	0.4	2.0	13.8
95th Queue (m)	12.4	4.6	10.0	40.5
Link Distance (m)	124.3	1.9		30.4
Upstream Blk Time (%)		0	0	3
Queuing Penalty (veh)		0	0	17
Storage Bay Dist (m)			15.0	
Storage Blk Time (%)			0	7
Queuing Penalty (veh)			1	1

Intersection: 3: Sandwich Street South & North Street/165 Sandwich Street South

Movement	EB	WB	NB	NB	SB	SB
Directions Served	LTR	LTR	L	TR	L	TR
Maximum Queue (m)	10.5	5.4	16.7	4.8	7.2	4.2
Average Queue (m)	3.9	0.3	5.8	0.2	8.0	0.2
95th Queue (m)	11.2	3.0	15.1	2.5	4.9	2.0
Link Distance (m)	118.7	50.2		79.6		93.0
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (m)			10.0		30.0	
Storage Blk Time (%)			5	0		
Queuing Penalty (veh)			24	0		

Intersection: 4: Sandwich Street South & Pedestrian Crossing

Movement	NB	SB
Directions Served	T	Т
Maximum Queue (m)	28.1	39.2
Average Queue (m)	22.7	28.6
95th Queue (m)	28.7	35.7
Link Distance (m)	5.6	14.1
Upstream Blk Time (%)	40	39
Queuing Penalty (veh)	182	210
Storage Bay Dist (m)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 5: Sandwich Street South & Access #1

Movement	SB
Directions Served	Т
Maximum Queue (m)	26.8
Average Queue (m)	16.4
95th Queue (m)	28.1
Link Distance (m)	1.9
Upstream Blk Time (%)	12
Queuing Penalty (veh)	62
Storage Bay Dist (m)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 6: Sandwich Street South & Site Access #2

Movement	WB	NB	SB	SB
Directions Served	LR	TR	L	Т
Maximum Queue (m)	8.9	52.1	2.2	21.8
Average Queue (m)	0.4	19.6	0.1	16.5
95th Queue (m)	3.5	42.4	1.1	20.4
Link Distance (m)	26.3	93.0		5.6
Upstream Blk Time (%)			0	0
Queuing Penalty (veh)			0	1
Storage Bay Dist (m)			5.0	
Storage Blk Time (%)			0	0
Queuing Penalty (veh)			0	0

Network Summary

Network wide Queuing Penalty: 499

Intersection: 1: Sandwich Street South & Fort Street

Movement	WB	WB	NB	SB	SB
Directions Served	L	R	TR	L	T
Maximum Queue (m)	21.7	19.7	9.6	10.6	6.4
Average Queue (m)	7.9	6.9	0.4	2.6	0.2
95th Queue (m)	16.6	16.0	4.2	9.4	4.5
Link Distance (m)		228.3	30.4		61.8
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (m)	20.0			30.0	
Storage Blk Time (%)	1	0			0
Queuing Penalty (veh)	0	0			0

Intersection: 2: Sandwich Street South & Parking Lot Driveway

Movement	WB	NB	SB	SB
Directions Served	LR	TR	L	Т
Maximum Queue (m)	14.2	14.5	9.3	23.1
Average Queue (m)	4.7	2.8	1.1	2.2
95th Queue (m)	12.6	11.9	6.0	14.5
Link Distance (m)	124.3	1.4		30.4
Upstream Blk Time (%)		0		0
Queuing Penalty (veh)		1		2
Storage Bay Dist (m)			15.0	
Storage Blk Time (%)			0	1
Queuing Penalty (veh)			0	0

Intersection: 3: Sandwich Street South & North Street/165 Sandwich Street South

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	L	L
Maximum Queue (m)	14.2	10.2	8.9	8.8
Average Queue (m)	3.1	2.1	0.6	0.6
95th Queue (m)	10.7	8.5	4.3	4.2
Link Distance (m)	118.7	50.2		
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (m)			10.0	30.0
Storage Blk Time (%)			0	
Queuing Penalty (veh)			2	

Intersection: 4: Sandwich Street South & Pedestrian Crossing

Movement	NB	SB
Directions Served	T	Т
Maximum Queue (m)	20.4	24.2
Average Queue (m)	2.0	3.2
95th Queue (m)	12.7	16.3
Link Distance (m)	7.2	12.9
Upstream Blk Time (%)	1	2
Queuing Penalty (veh)	10	13
Storage Bay Dist (m)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 5: Sandwich Street South & Access #1

Movement	WB	NB	SB
Directions Served	LR	T	T
Maximum Queue (m)	15.1	10.0	15.8
Average Queue (m)	4.2	0.3	1.6
95th Queue (m)	13.1	4.2	9.9
Link Distance (m)	36.4	12.9	1.4
Upstream Blk Time (%)		0	0
Queuing Penalty (veh)		0	1
Storage Bay Dist (m)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 6: Sandwich Street South & Access #2

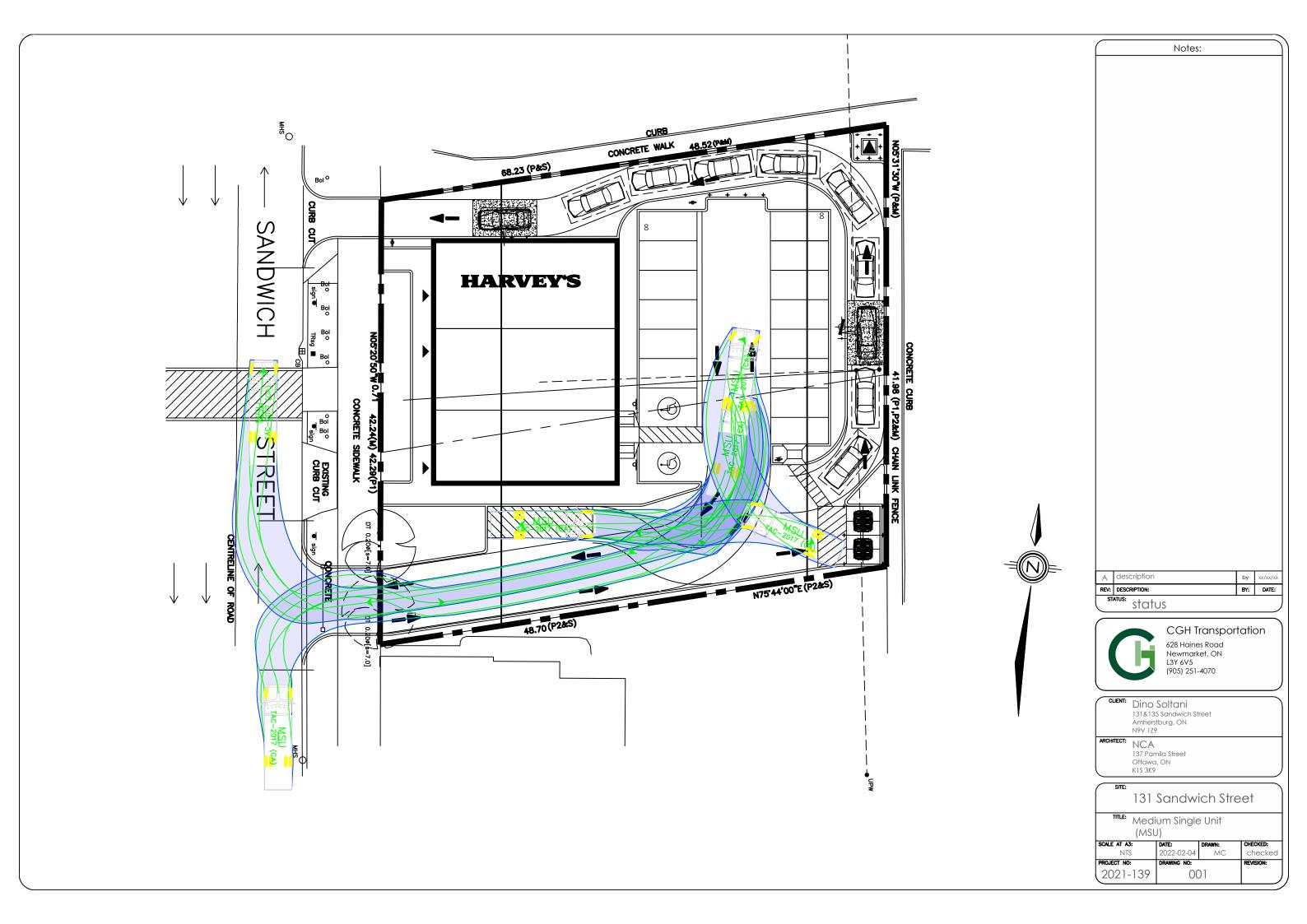
Movement	WB	NB	SB	SB
Directions Served	LR	TR	L	Т
Maximum Queue (m)	10.3	29.7	7.0	13.6
Average Queue (m)	2.6	2.6	1.6	2.3
95th Queue (m)	9.4	17.5	6.3	9.4
Link Distance (m)	43.0	93.1		7.2
Upstream Blk Time (%)			1	0
Queuing Penalty (veh)			0	3
Storage Bay Dist (m)			5.0	
Storage Blk Time (%)			1	0
Queuing Penalty (veh)			7	0

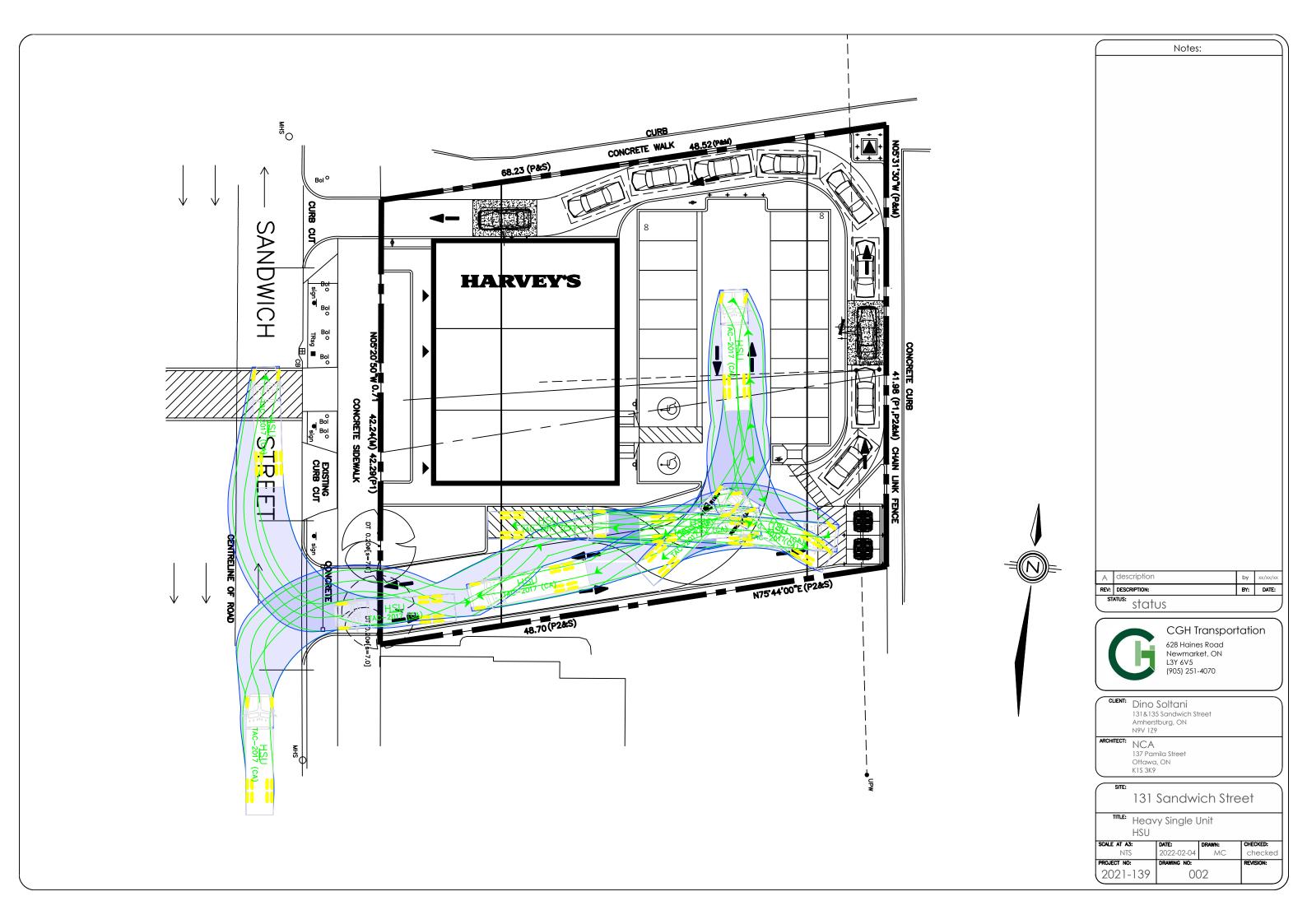
Network Summary

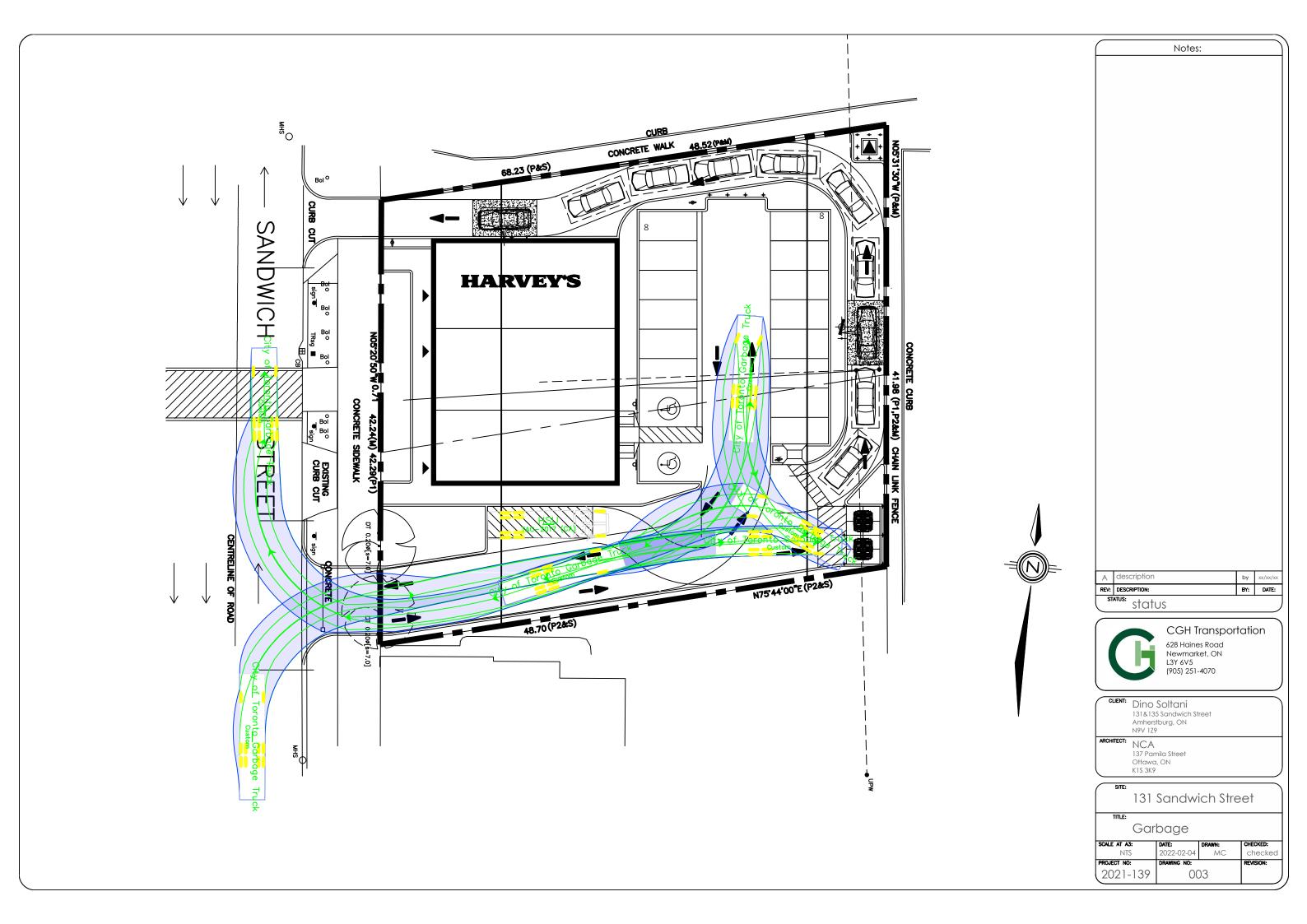
Network wide Queuing Penalty: 40

Appendix I

Turning Templates







Appendix J

Parking Count Survey

Ontario Traffic Inc - Parking Counts

Location: Sandwich Street S

Date: Thursday, February 24, 2022

	Time		Parked Vehicles
11:30		12:00	54
	to		
12:00	to	12:30	55
12:30	to	13:00	56
13:00	to	13:30	55

16:00	to	16:30	6
16:30	to	17:00	4
17:00	to	17:30	3
17:30	to	18:00	2
Avai	153		